CITY OF DAYTON Public Works Design Standards

Standard Detail Drawings & Sample Test Report Forms

Appendix A

Note

- 1) Per PWDS 1.11.b.12, the applicable City standard details shall be included on construction drawings submitted for City review and approval. See also PWDS 1.3.a.3 for detail sheet stamping requirements where engineered drawings are required.
- 2) Per PWDS 1.2.b, the City standard details are intended to assist but not to substitute for competent work by design professionals where applicable. As noted in the PWDS, the City standard details illustrate the minimum requirements and materials required by the Public Works Department for the construction of certain standard system components, and are thus not considered to be final documents until incorporated into a design approved by the City,

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CITY OF DAYTON PUBLIC WORKS DESIGN STANDARDS

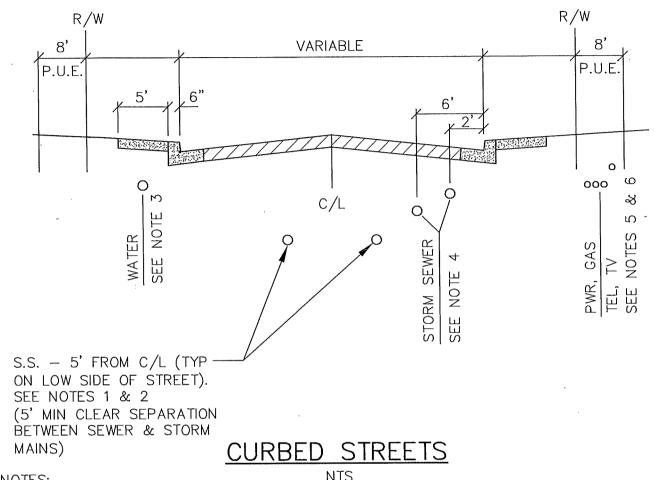
TABLE OF CONTENTS, CITY STANDARD DETAILS

• <u>A1 -</u>	General Requirements	Latest Revision
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- A3	- Trenches & Stormwater Management	Latest Revision
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,	For storm sewer service laterals, see Details 401 – 403A For storm sewer service laterals, see Details 415 & 416	
,	For storm sewer service faterals, see Details 413 & 410	
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305 308	Bore Casing, Carrier Pipe & Casing Spacer Detail	
500	Dore Cusing, Currer ripe to Cusing Spacer Dount	
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٠, ٥,٠	Test Reports	
>	Storm Drain Mandrel Test Report	
	Storm Drain Pineline TV Inspection Report	

• A4	Sanitary Sewers	Latest Revision
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•	For bore casing detail, see Detail 308	
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419	Inserta-Tee Connection to Existing Sewer or Storm Drain	1/24
	Test Reports	
	Sanitary Sewer Manhole Vacuum Test Report	
	Sanitary Sewer Air Test Report	
)	Sanitary Sewer Mandrel Test Report	
	Sanitary Sewer Pipeline TV Inspection Report	
•	Samual y Sewer 1 ipenite 1 v inspection report	
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• A > -	_ Water Distribilition	Latest Revision
• <u>A3</u>	- Water Distribution For trench backfill and surface restoration, see Details 301 - 304	Latest Revision
• <u>A5</u> -	For trench backfill and surface restoration, see Details 301 - 304	Latest Revision
<u>A5-</u>		Latest Revision
)	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308	
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501 502	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24
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501 502 503	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24 9/24
501 502 503	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24 9/24 9/18 7/24
501 502 503	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24 9/24 9/18 7/24
501 502 503 505 506 507	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24 9/24 9/18 7/24
501 502 503 505 506 507 510	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	7/24 6/24 9/24 9/18 7/24 7/24
501 502 503 505 506 507 510 511	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
501 502 503 505 506 507 510 511 512	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
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501 502 503 505 506 507 510 511 512 513A 513B	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
501 502 503 505 506 507 510 511 512 513A	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
501 502 503 505 506 507 510 511 512 513A 513B	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail Butterfly Valve and Valve Box Detail Standard Fire Hydrant Assembly Tapping Tee and Valve Mainline Blowoff Assembly Standard Blowoff with Plugged End Horizontal Thrust Blocking Straddle Block for 12" & Smaller Pipe Vertical Thrust Blocking New Waterline Under-Crossing (to Accommodate New/Existing Utility Conflict) Existing Water Service 1½" and 2" Meter Set with 1" High Bypass Connection Requirements, 1½" and 2" Service (HDPE or Sched 80 PVC Service Line)	
501 502 503 505 506 507 510 511 512 513A 513B	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
501 502 503 505 506 507 510 511 512 513A 513B 515 516 517A	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail Butterfly Valve and Valve Box Detail Standard Fire Hydrant Assembly Tapping Tee and Valve Mainline Blowoff Assembly Standard Blowoff with Plugged End Horizontal Thrust Blocking Straddle Block for 12" & Smaller Pipe Vertical Thrust Blocking New Waterline Under-Crossing (to Accommodate New/Existing Utility Conflict) Existing Waterline Lowering (to Accommodate New Storm, Sewer, etc.) Typical 1" Water Service 1½" and 2" Meter Set with 1" High Bypass Connection Requirements, 1½" and 2" Service (HDPE or Sched 80 PVC Service Line) Connection Requirements, 3" & Larger Service	
501 502 503 505 506 507 510 511 512 513A 513B 515 516 517A 517B	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail Butterfly Valve and Valve Box Detail Standard Fire Hydrant Assembly Tapping Tee and Valve Mainline Blowoff Assembly Standard Blowoff with Plugged End Horizontal Thrust Blocking Straddle Block for 12" & Smaller Pipe Vertical Thrust Blocking New Waterline Under-Crossing (to Accommodate New/Existing Utility Conflict) Existing Waterline Lowering (to Accommodate New Storm, Sewer, etc.) Typical 1" Water Service 1½" and 2" Meter Set with 1" High Bypass Connection Requirements, 1½" and 2" Service (HDPE or Sched 80 PVC Service Line) Connection Requirements, 3" & Larger Service 1" Combination Air Release Valve (CARV)	
501 502 503 505 506 507 510 511 512 513A 513B 515 516 517A 517B 518	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail	
501 502 503 505 506 507 510 511 512 513A 513B 515 516 517A 517B 518 519	For trench backfill and surface restoration, see Details 301 - 304 For bore casing detail, see Detail 308 Gate Valve and Valve Box Detail Butterfly Valve and Valve Box Detail Standard Fire Hydrant Assembly Tapping Tee and Valve Mainline Blowoff Assembly Standard Blowoff with Plugged End Horizontal Thrust Blocking Straddle Block for 12" & Smaller Pipe Vertical Thrust Blocking New Waterline Under-Crossing (to Accommodate New/Existing Utility Conflict) Existing Waterline Lowering (to Accommodate New Storm, Sewer, etc.) Typical 1" Water Service 1½" and 2" Meter Set with 1" High Bypass Connection Requirements, 1½" and 2" Service (HDPE or Sched 80 PVC Service Line) Connection Requirements, 3" & Larger Service	

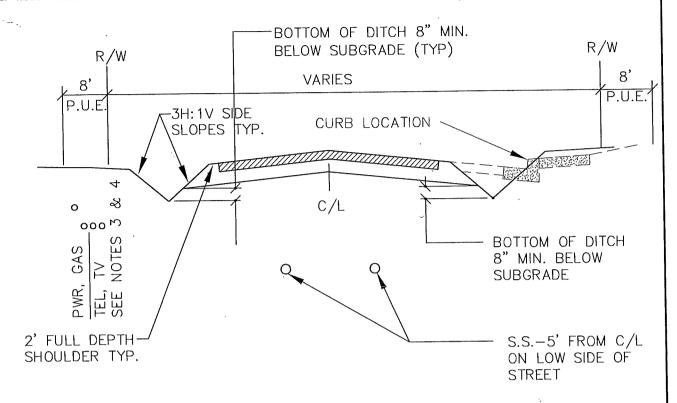
526	8" Domestic Water Meter	4/24
527	Water Meter Vault Bypass Valve Lock	6/25
528	Water Meter Test Port Assembly	4/24
529	Water Meter Test Port Assembly	1/18
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554	4" Double Check Detector Assembly w/FDC	1/24
555	6" Double Check Detector Assembly w/FDC	1/24
556	8" Double Check Detector Assembly w/FDC	1/24
559	4" Forward Flow Test Port inside DCDA Vault (For NFPA 13 & 25 Tests)	1/24
560	Below Grade Check Valve & Ball Drip Valve, in Close Bottom Drain Structure	8/22
561	Below Grade Check Valve & Ball Drip Valve, in Open Bottom Drain Structure	4/25
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563	FDC on Building Exterior & Forward Flow Test Port, Sample & Notes	9/24
	Test Reports	
•	Waterline Pressure Test Report	
• <u>A6</u>	- Erosion Control	
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611	Sediment Barriers	4/14
612	Straw Wattle Sediment Barrier	6/15
613	Inlet Sediment Control	4/14
614	Ditch & Swale Protection	4/14
615	Silt Sack Inlet Detail	9/06
616	Temporary Concrete Washout Area (CWA)	11/18
617	Stockpile Cover Detail	1/19



- 1. 6' MIN COVER TYPICALLY REQUIRED FOR SANITARY SEWER MAINS (4' MIN. COVER TYPICALLY REQUIRED FOR LATERALS).
- 2. LATERALS AND P/L CLEANOUTS SHALL BE INSTALLED DURING CONSTRUCTION OF SANITARY SEWER & STORM MAINS (TO AVOID FUTURE STREET CUTS).
- 3. WATER TO BE INSTALLED 3' BEHIND FACE OF CURB ON HIGH SIDE OF STREET. MIN. COVER ON ALL WATERLINES. 10' MINIMUM SEPARATION TYPICAL BETWEEN PARALLEL WATER & SEWER MAINS.
- 4. STORM SEWER TO BE INSTALLED ON LOW SIDE OF STREET:
 - a) 2' FROM FACE OF CURB FOR <4' RIM TO INVERT
 - b) 6' FROM FACE OF CURB FOR >4' RIM TO INVERT (MH SYSTEM)
- 5. MAINTAIN MIN. 5' HORIZ. SEPARATION BETWEEN PUBLIC UTILITIES & PARALLEL PRIVATE UTILITIES. OTHER VERTICAL AND HORIZONTAL SEPARATION DISTANCES SHALL BE AS SPECIFIED BY DEQ, ODWP, OR PUBLIC/PRIVATE UTILITY COMPANIES.
- 6. UNITY TRENCH PER FRANCHISE UTILITY COMPANY REQUIREMENTS, GENERALLY ON OPPOSITE SIDE OF STREET FROM WATER LINE WHERE FEASIBLE.

LAST REVISION DATE: FEB 2024	COPYRIGHT 1996 WESTECH ENGINEERING, INC.	
TYP. UTILITY LOCATIONS (CURBED STREETS)		
· (N	TS)	
DAYTON OP	DETAIL NO.	

UTILITIES FOR 3/4 STREET IMPROVEMENTS OR EXISTING TURNPIKE STREETS SHALL BE LOCATED TO ALLOW FUTURE CONSTRUCTION OF CURBED STREETS WITHOUT RELOCATING UTILITIES. SEE DETAIL 101.



TURNPIKE STREETS

NOTES:

NTS

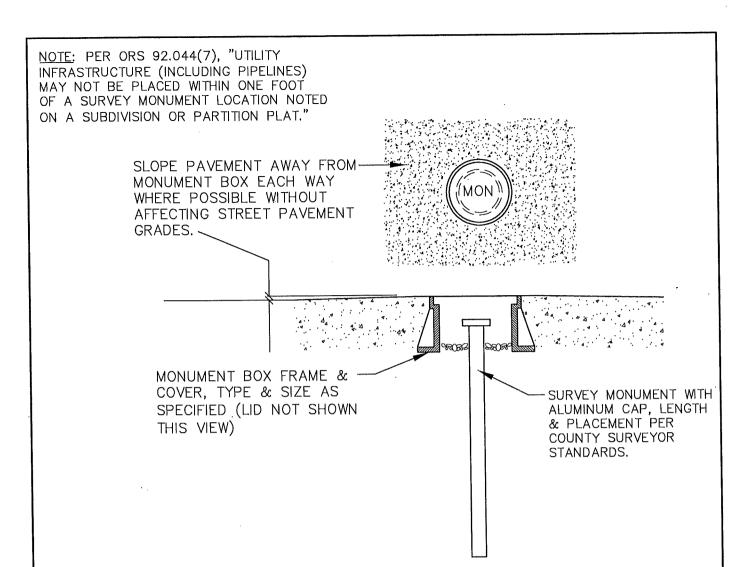
- 1. 6' MIN COVER TYPICALLY REQUIRED FOR SANITARY SEWER MAINS (4' MIN. COVER TYPICALLY REQUIRED FOR LATERALS).
- 2. LATERALS AND P/L CLEANOUTS SHALL BE INSTALLED DURING CONSTRUCTION OF SANITARY SEWER & STORM MAINS (TO AVOID FUTURE STREET CUTS).
- 3. WATER TO BE INSTALLED 3' BEHIND FACE OF CURB ON IMPROVED SIDE OR 3' BEHIND FUTURE FACE OF CURB LOCATION AS DIRECTED BY THE CITY ENGINEER. 10' MINIMUM SEPARATION TYPICAL BETWEEN PARALLEL WATER & SEWER MAINS.
- 4. MAINTAIN MIN. 5' HORIZ. SEPARATION BETWEEN PUBLIC UTILITIES & PARALLEL PRIVATE UTILITIES. OTHER VERTICAL AND HORIZONTAL SEPARATION DISTANCES SHALL BE AS SPECIFIED BY DEQ, ODWP, OR PUBLIC/PRIVATE UTILITY COMPANIES.
- 5. UNITY TRENCH PER FRANCHISE UTILITY COMPANY REQUIREMENTS, GENERALLY ON OPPOSITE SITE OF STREET FROM WATER LINE WHERE FEASIBLE.

LAST REVISION DATE:
FEB 2024

TYP. UTILITY LOCATIONS
(TURNPIKE AND 3/4 STREETS)

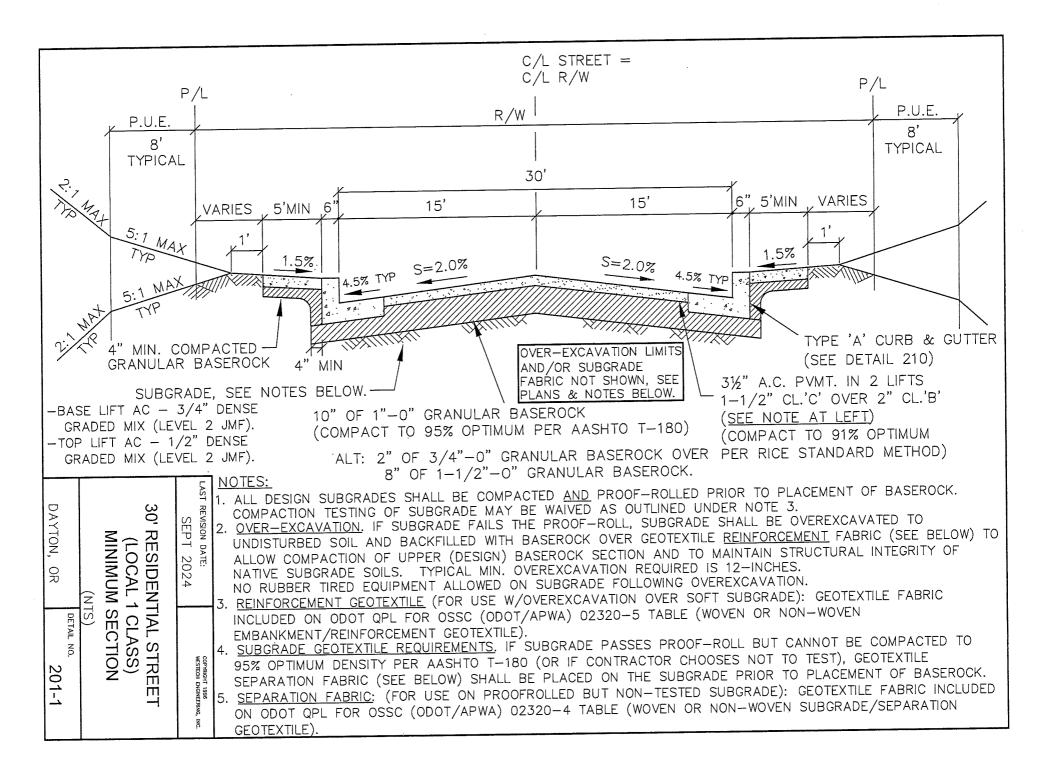
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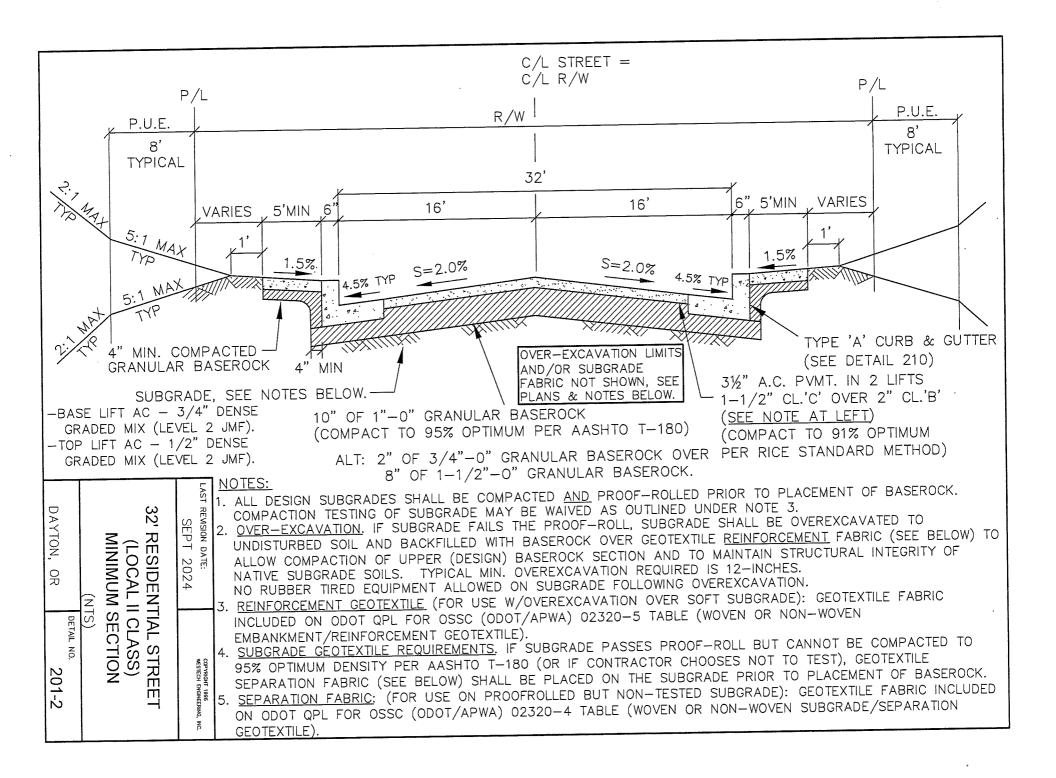
DAYTON, OR 102

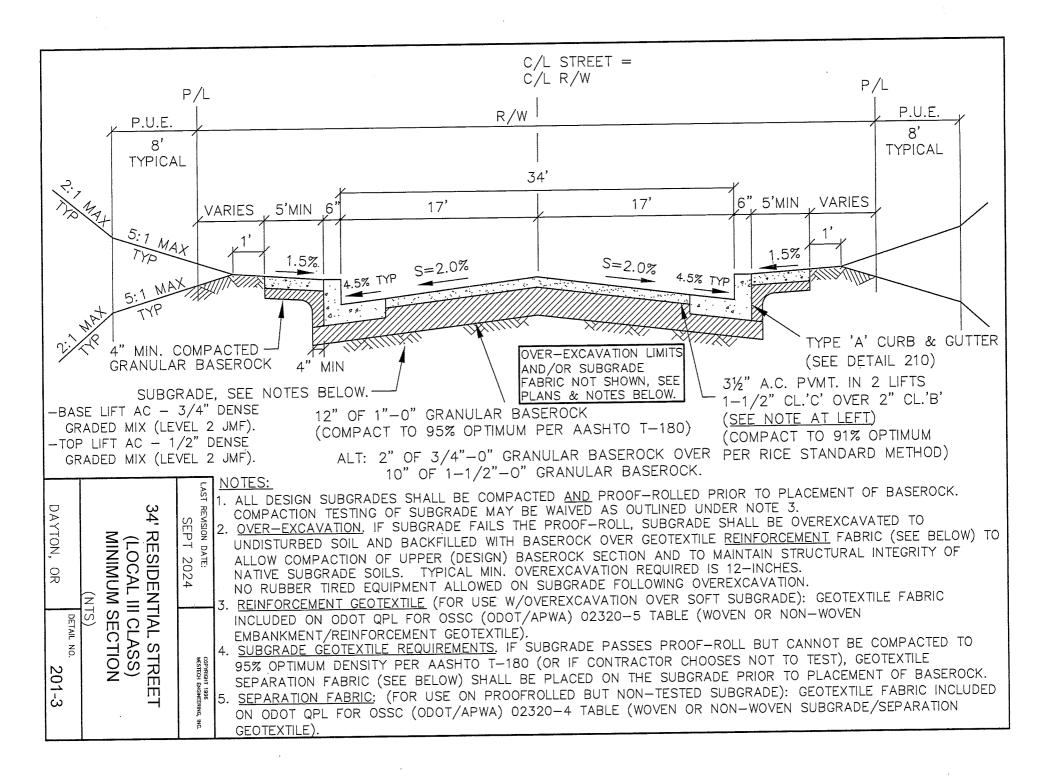


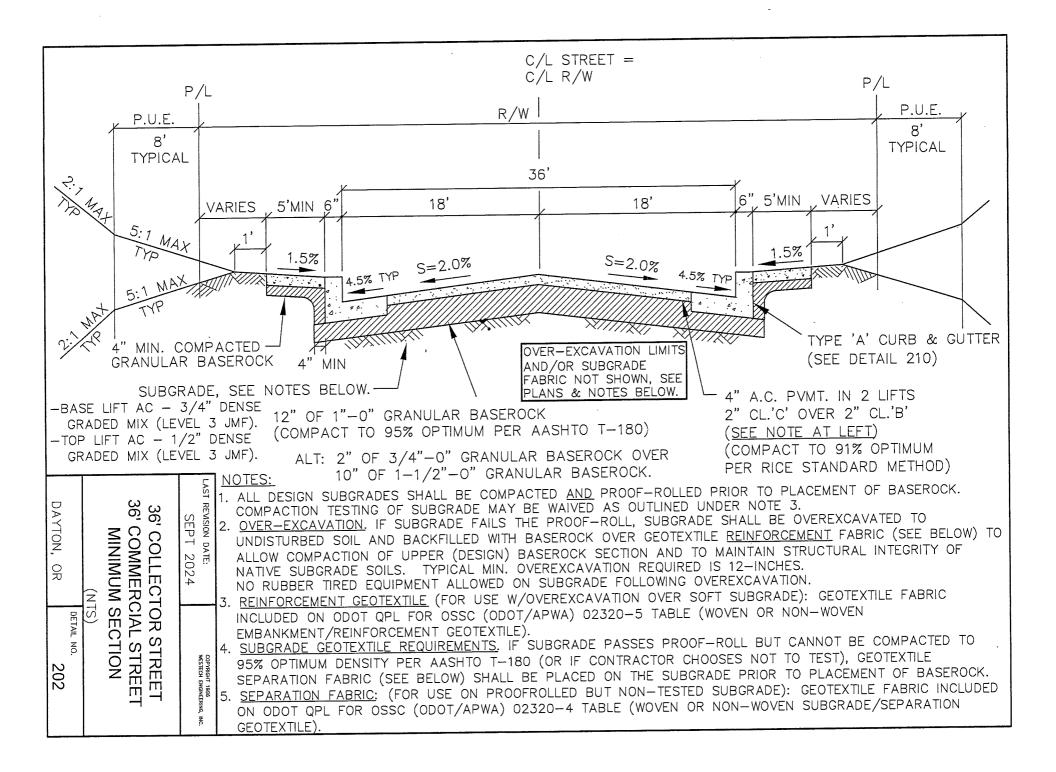
- 1. VERIFY MONUMENT BOX SIZE WITH COUNTY SURVEYOR PRIOR TO PLACEMENT. UNLESS OTHERWISE REQUIRED BY THE COUNTY SURVEYOR (BASED ON TYPE OF SURVEY MONUMENT), PROVIDE THE FOLLOWING.
 - a) USE <u>8" DIAMETER</u> (MINIMUM) MONUMENT BOX FOR POSTED <u>SPEEDS LESS THAN 35 MPH.</u> (OLYMPIC M1014 BOX/LID, OR EJ 3614Z BOX W/3614A LID).
 - b) USE <u>12" DIAMETER</u> MONUMENT BOX FOR POSTED <u>SPEEDS EQUAL TO OR GREATER THAN 35 MPH.</u> (EJ 3673Z BOX W/3673A LID).
- 2. FOR REPAVING PROJECTS, PROVIDE OVERLAY RISER RINGS FROM SAME MANUFACTURER, HEIGHT AS REQUIRED TO ACCOMODATE OVERLAY THICKNESS.

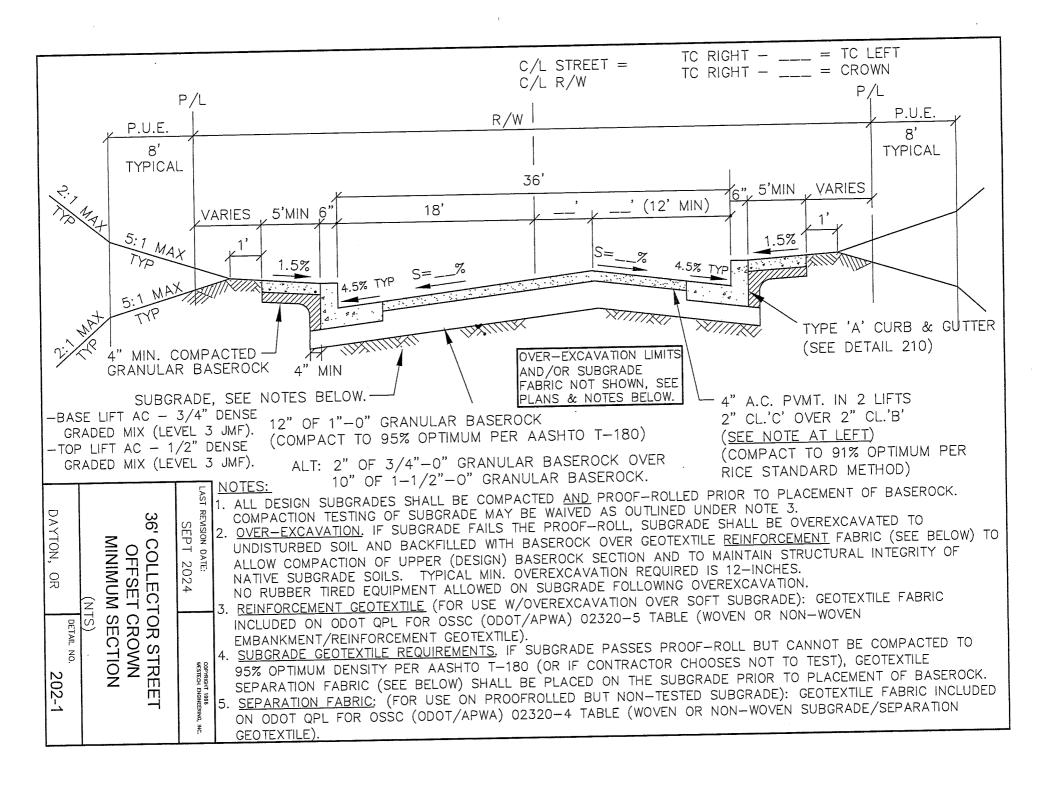
LAST REVISION DATE: SEPT 2020	COPYRICHT 1996 WESTECH ENGINEERING, INC.
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	DETAIL NO.

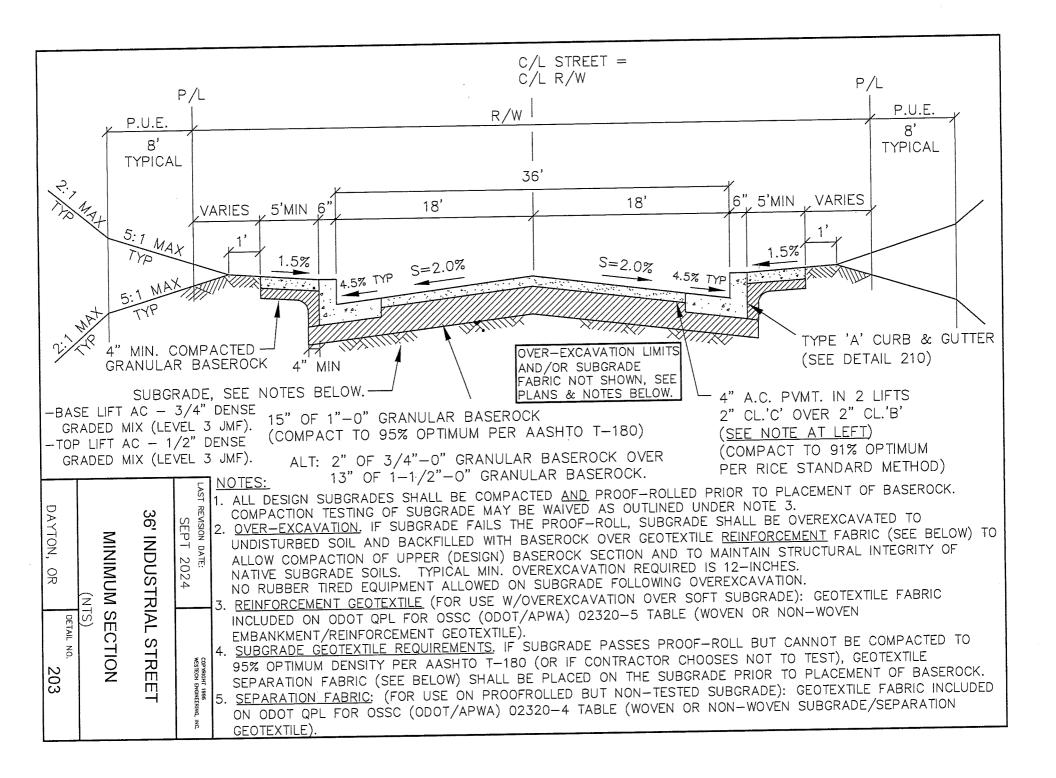


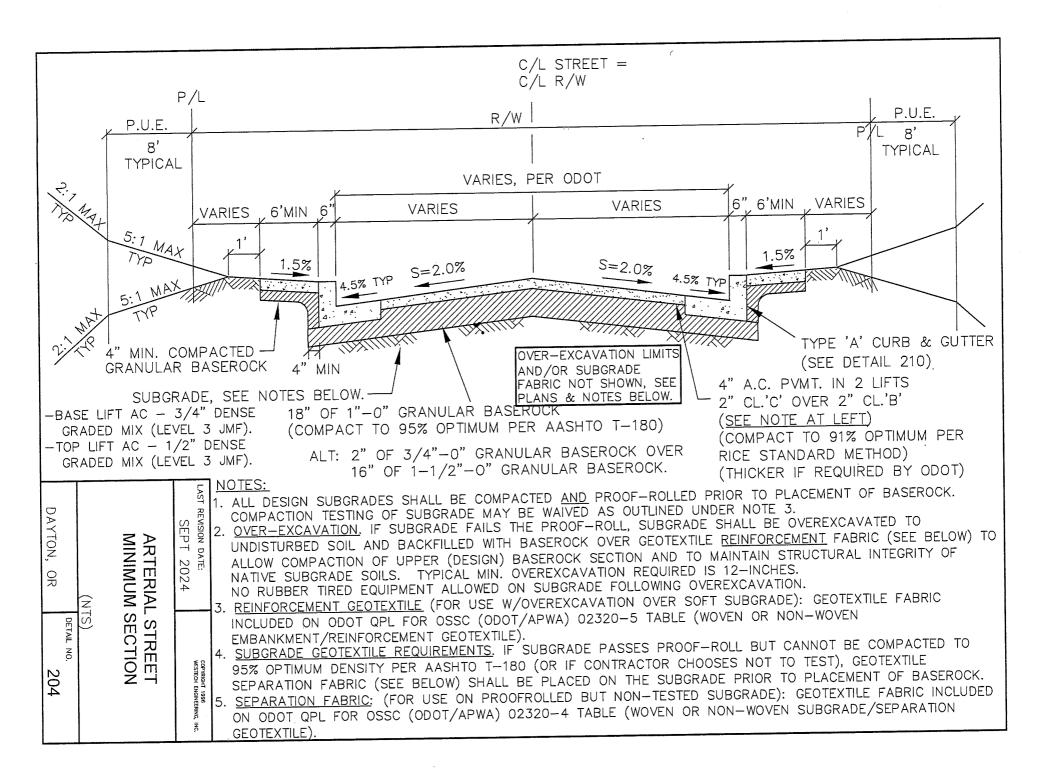


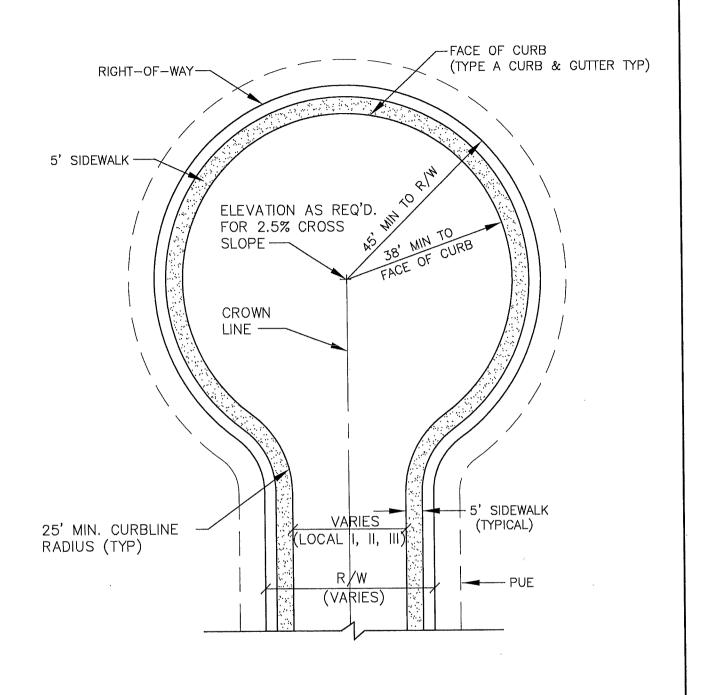






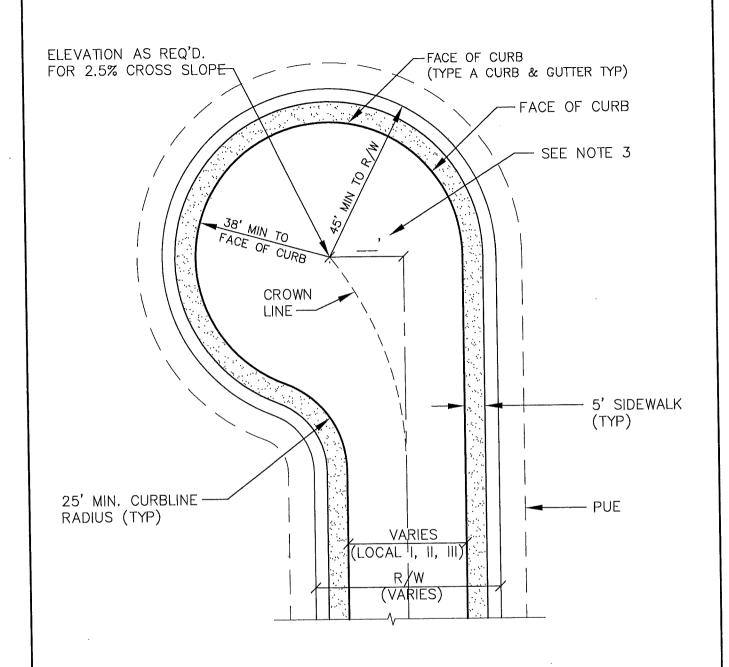






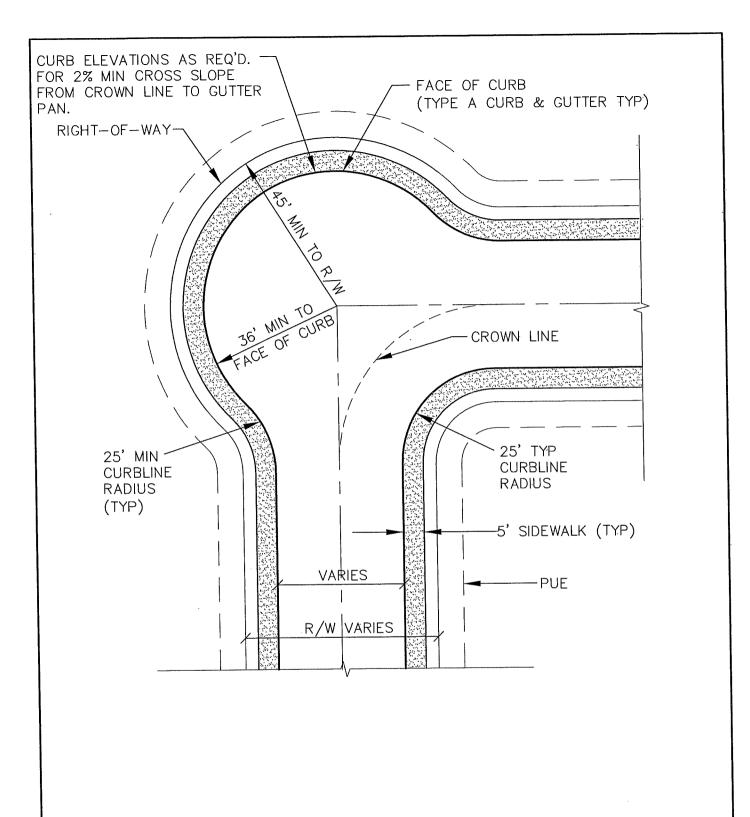
- 1. 2.5% MIN. CROSS SLOPE REQUIRED FROM CENTER OF BULB TO GUTTER.
- 2. MAINTAIN CROWN LINE TO CENTER OF CUL-DE-SAC BULB.

LAST REVISION DATE: DEC 2015	COPYRICHT 1996 WESTECH ENGINEERING, INC.	
STANDARD CUL-DE-SAC (RESIDENTIAL)		
(NTS)		
DAYTON, OR	DETAIL NO. 205	



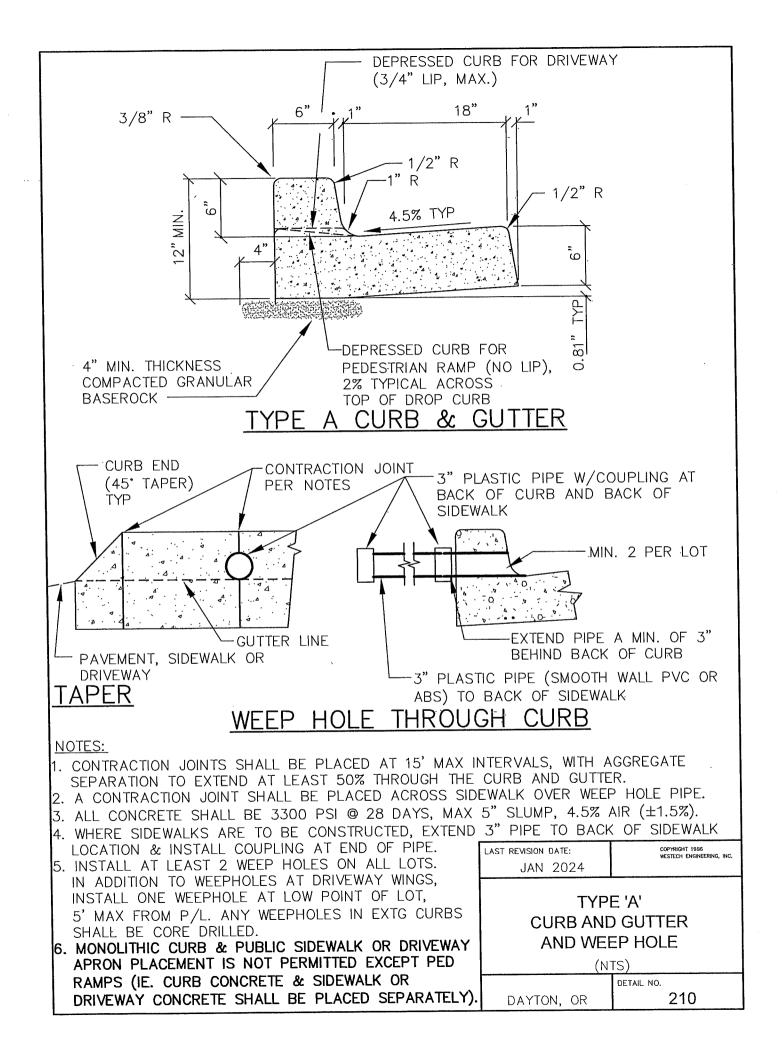
- 1. 2.5% MIN. CROSS SLOPE REQUIRED FROM CENTER OF BULB TO GUTTER.
- 2. MAINTAIN CROWN LINE TO CENTER OF CUL-DE-SAC BULB.
- 3. OFFSET FROM ROADWAY CENTERLINE TO CENTER OF BULB = CURB RADIUS MINUS ONE—HALF STREET WIDTH.

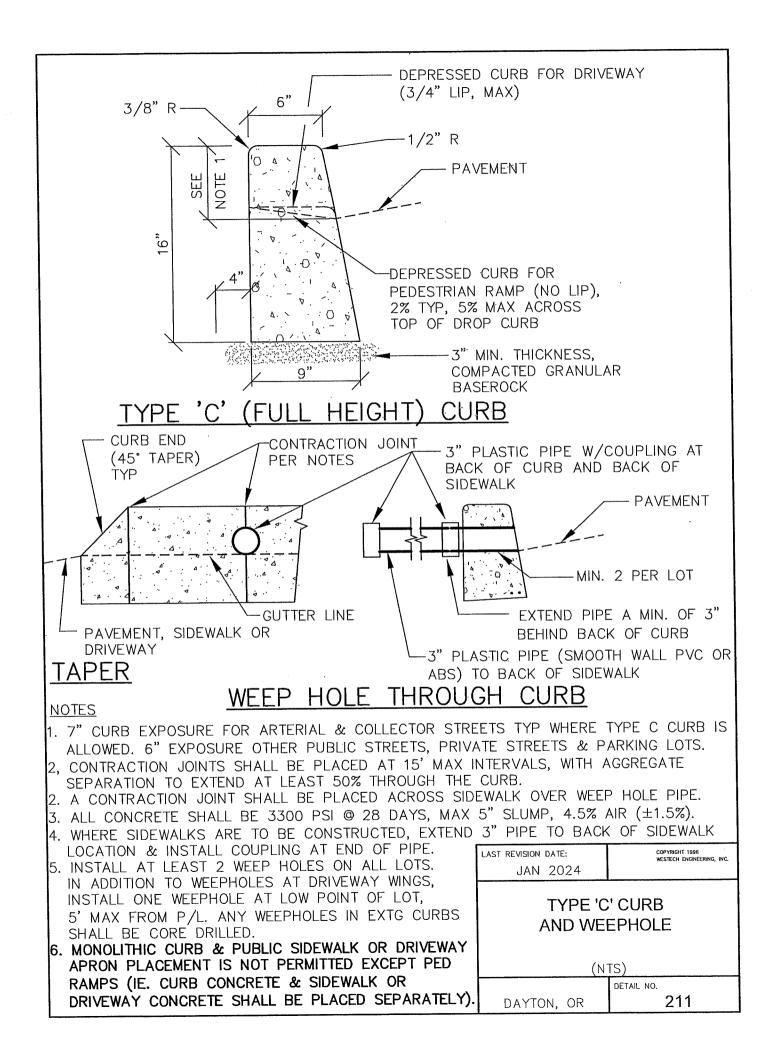
LAST REVISION DATE:	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
DEC 2015	
OFFSET CUL-DE-SAC (RESIDENTIAL) (NTS)	
DAYTON, OR	DETAIL NO. - 206

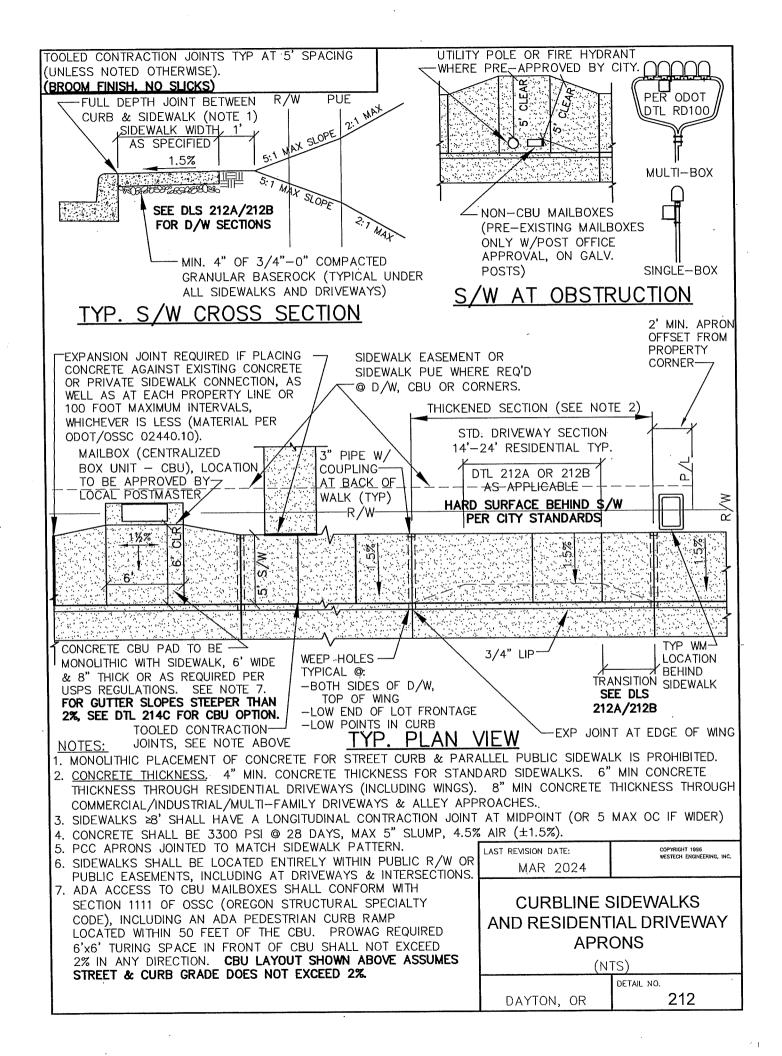


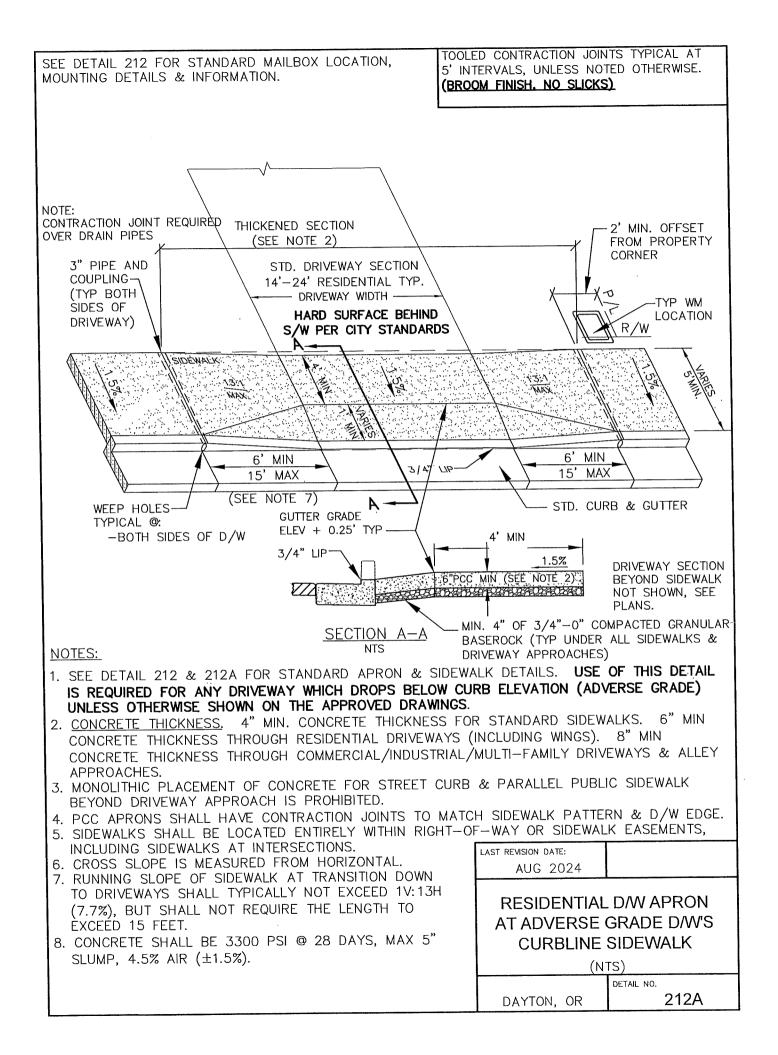
- 1. MAINTAIN CROWN LINE AROUND RADIUS OF CORNER AS SHOWN.
- 2. PROVIDE 2% MIN. CROSS SLOPE AS REQUIRED FROM CROWN LINE TO GUTTER.
- 3. PROVIDE CATCH BASIN IN EYEBROW CUL-DE-SAC IF REQUIRED FOR DRAINAGE COLLECTION.

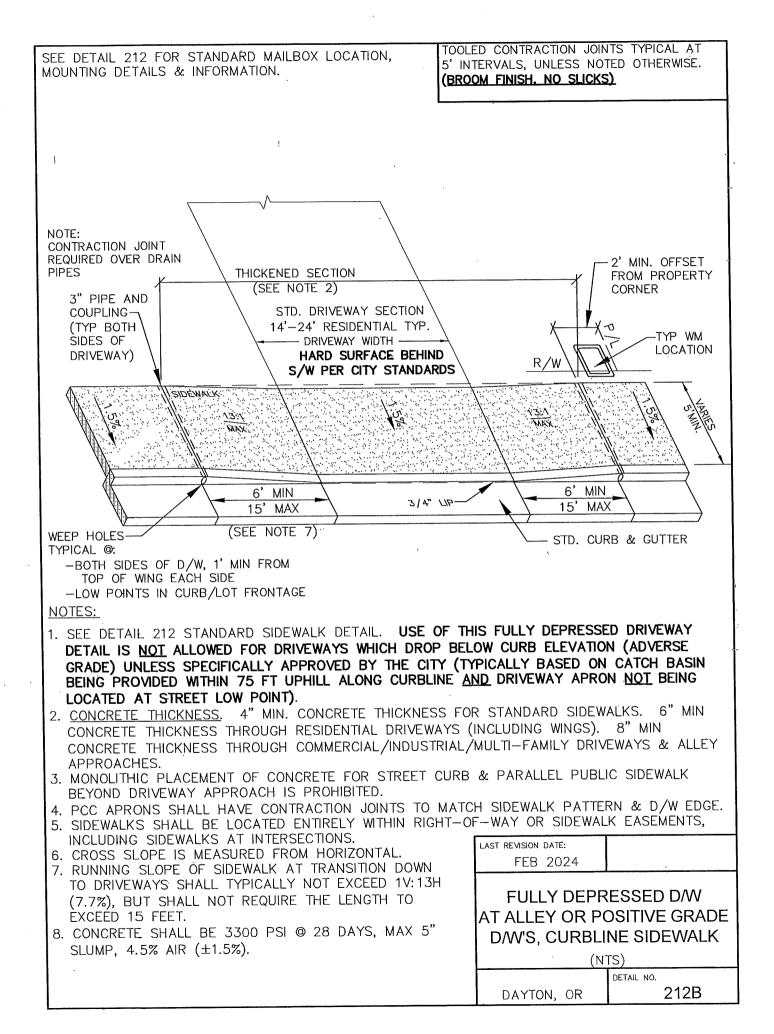
LAST REVISION DATE:	JO #	
JAN 2024		
EYEBROW CUL-DE-SAC (RESIDENTIAL) (NTS)		
	DETAIL NO.	
DAYTON, OR	207	

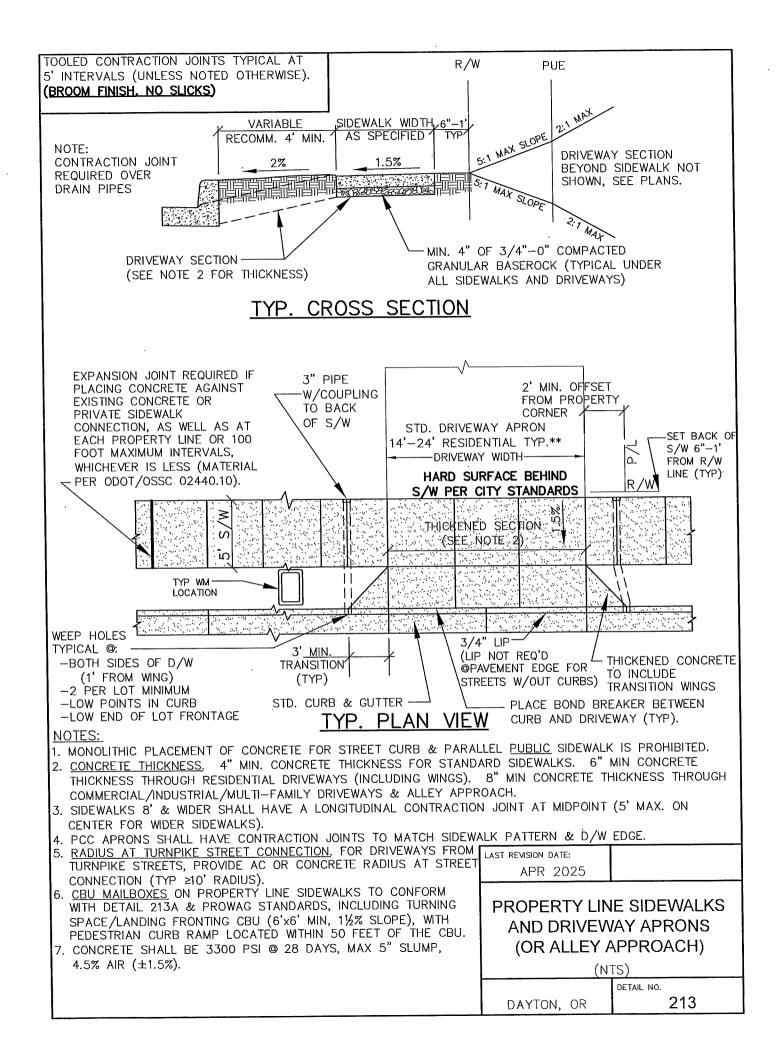


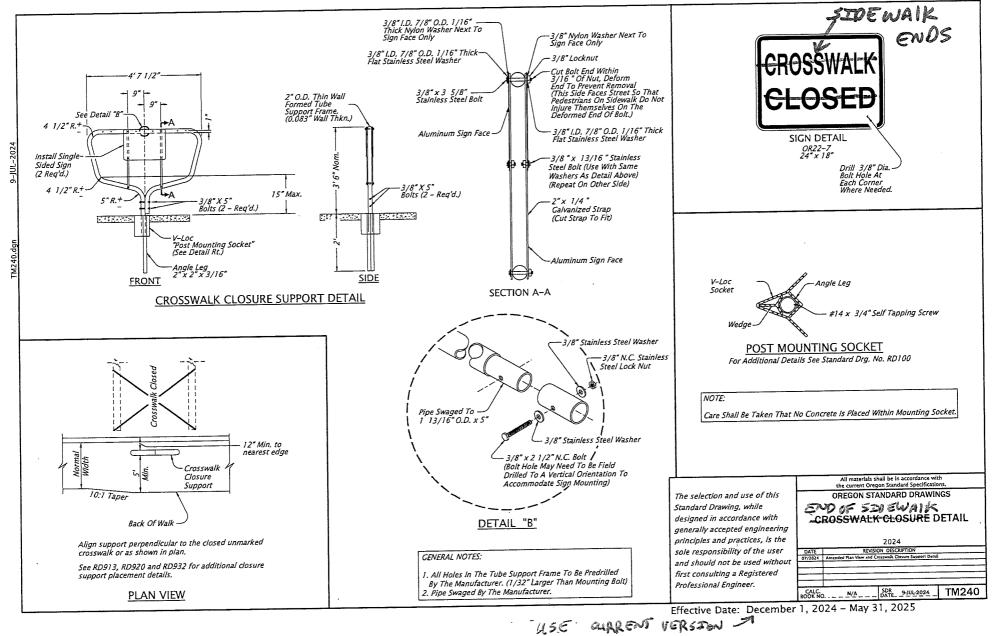


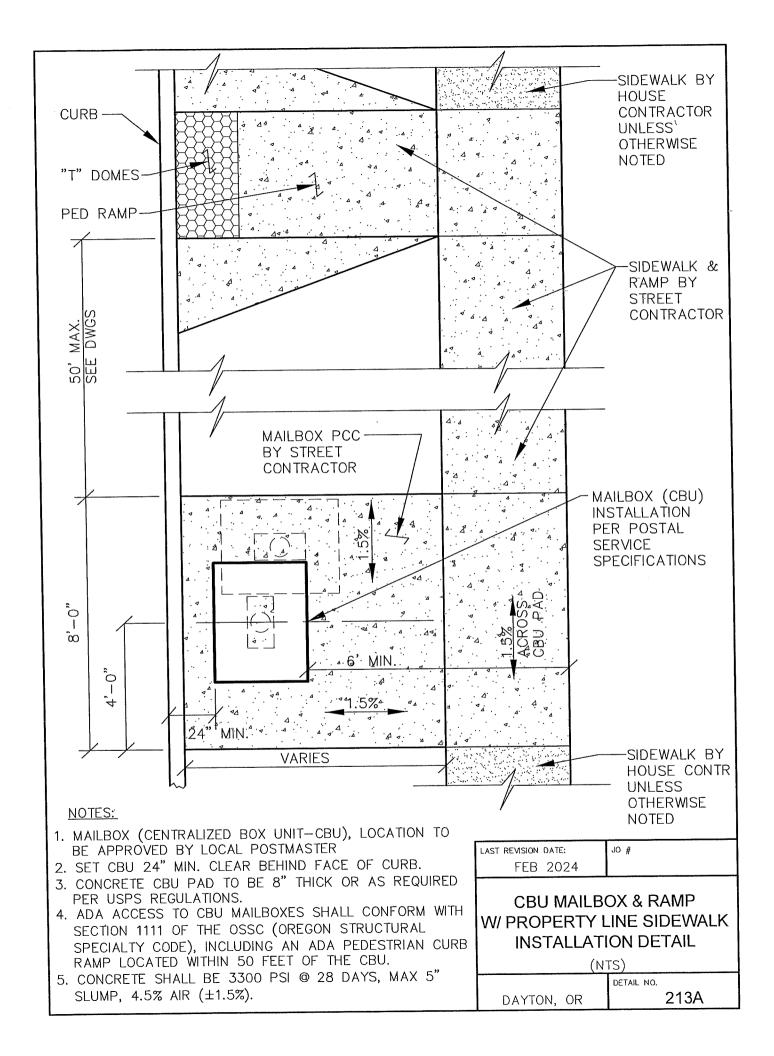


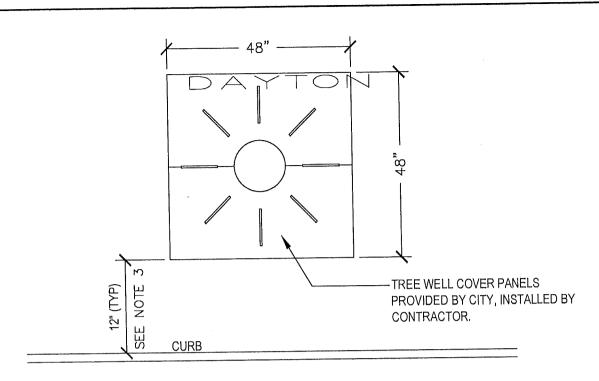


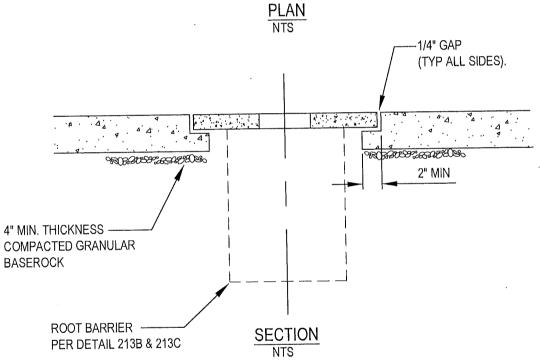










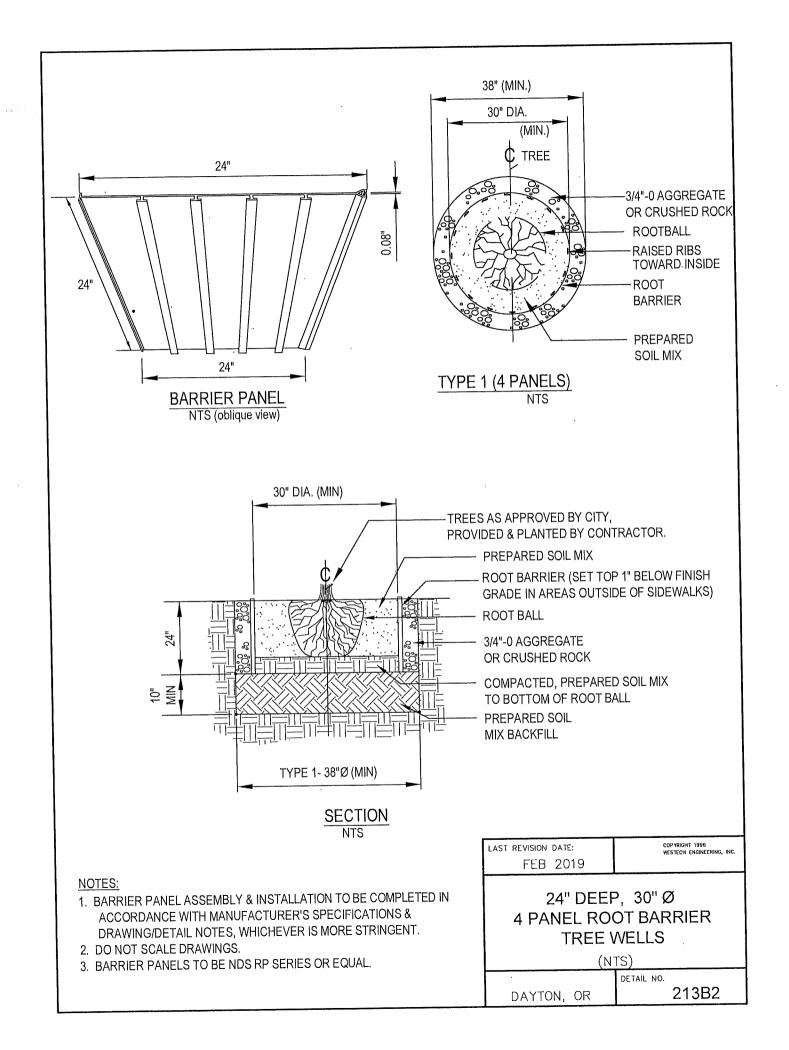


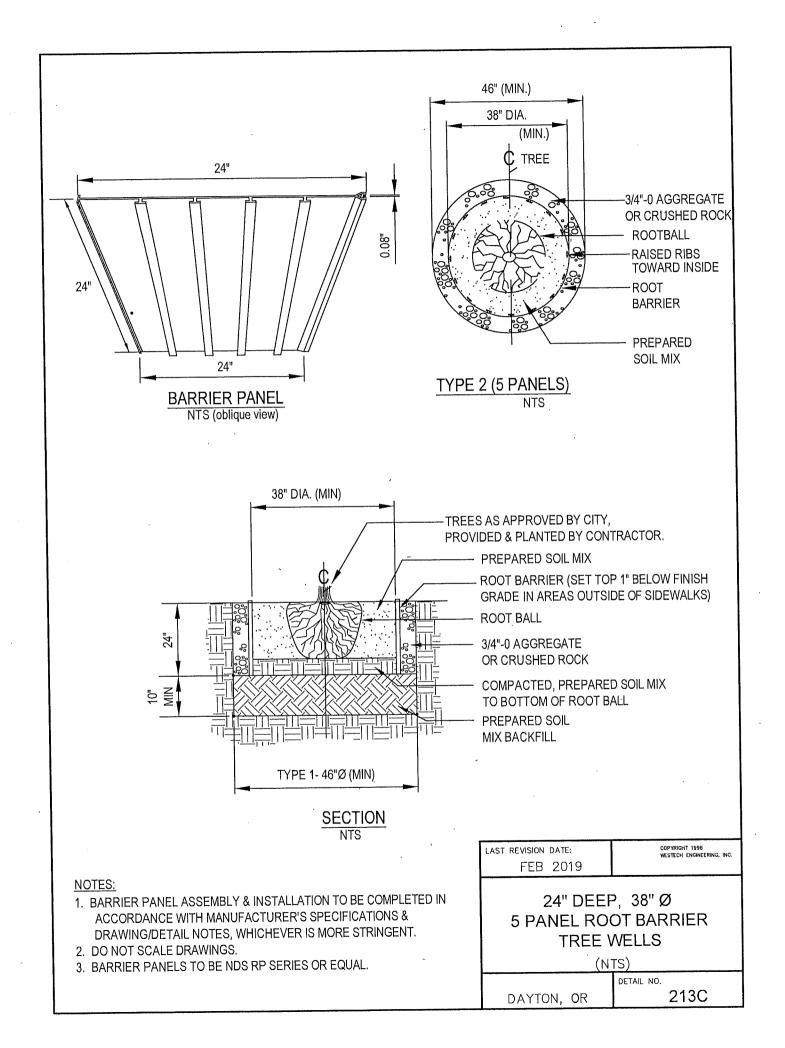
1. CONTRACTOR TO VERIFY INSET PANEL DIMENSIONS AND THICKNESS PRIOR TO FORMING BLOCKOUT AND LIP.

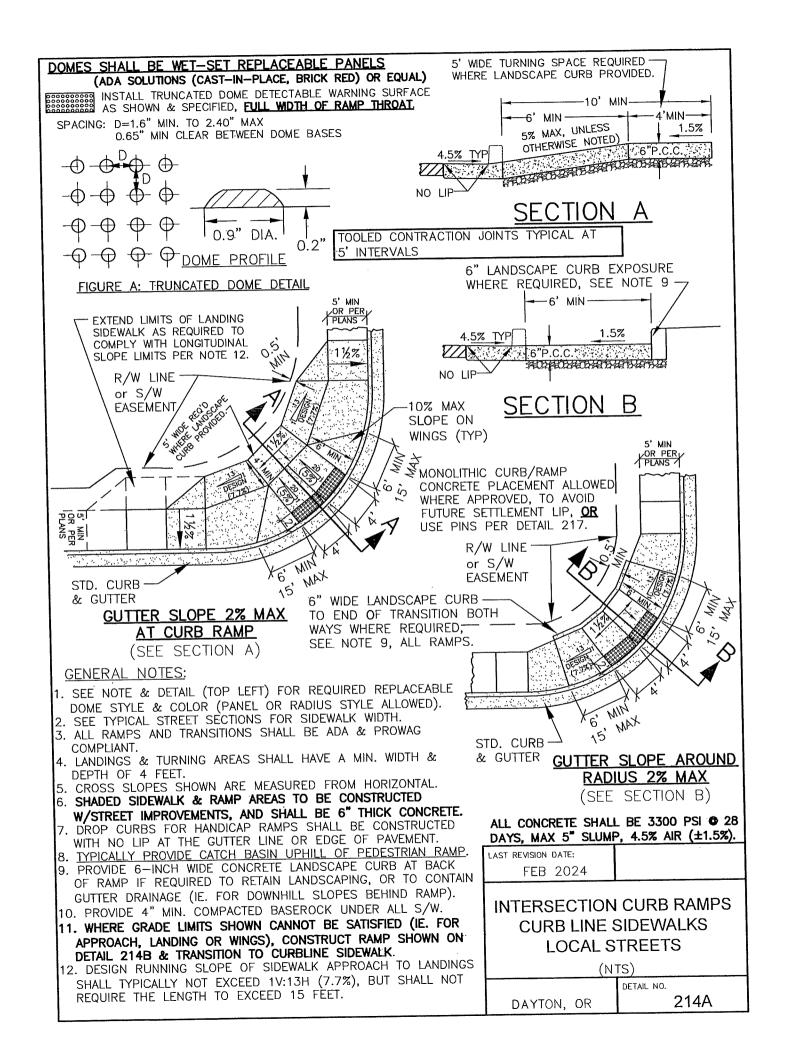
2. DRAWING NOT TO SCALE.

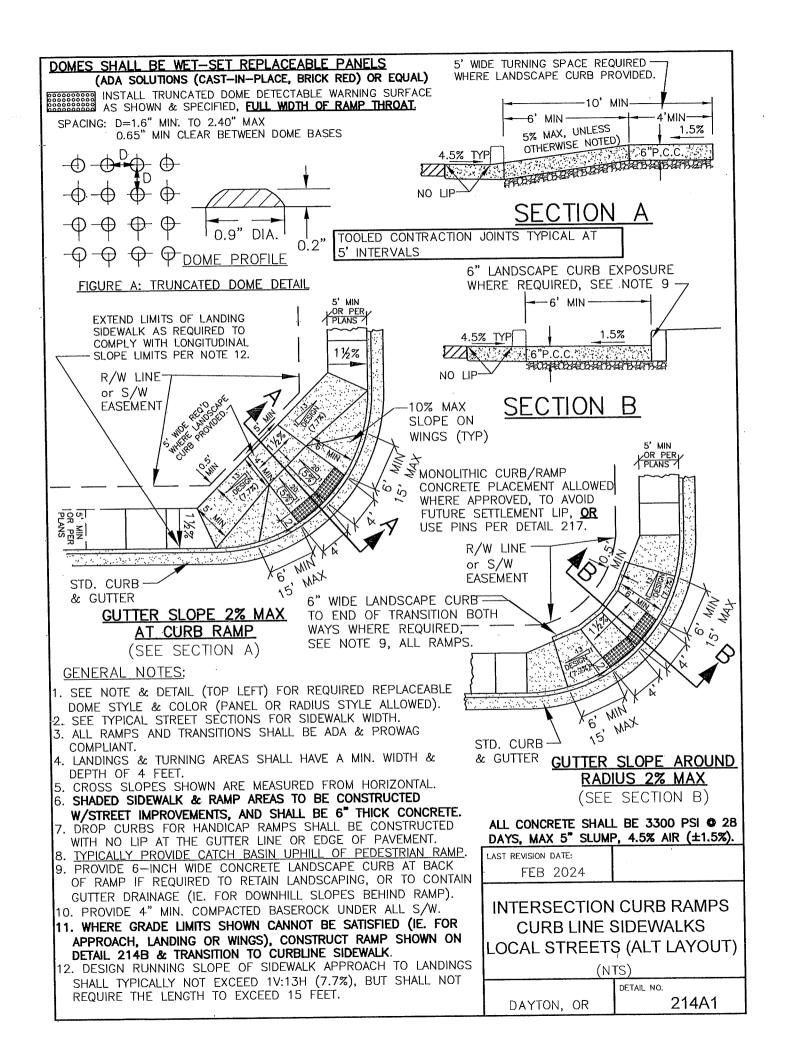
3. SPACING FROM CURB TO TREE WELL MAY VARY FOR SIDEWALKS NARROWER THAN 12 FOOT STANDARD FOR CBO ZONE (SEE DRAWINGS FOR ACTUAL DIMENSION).

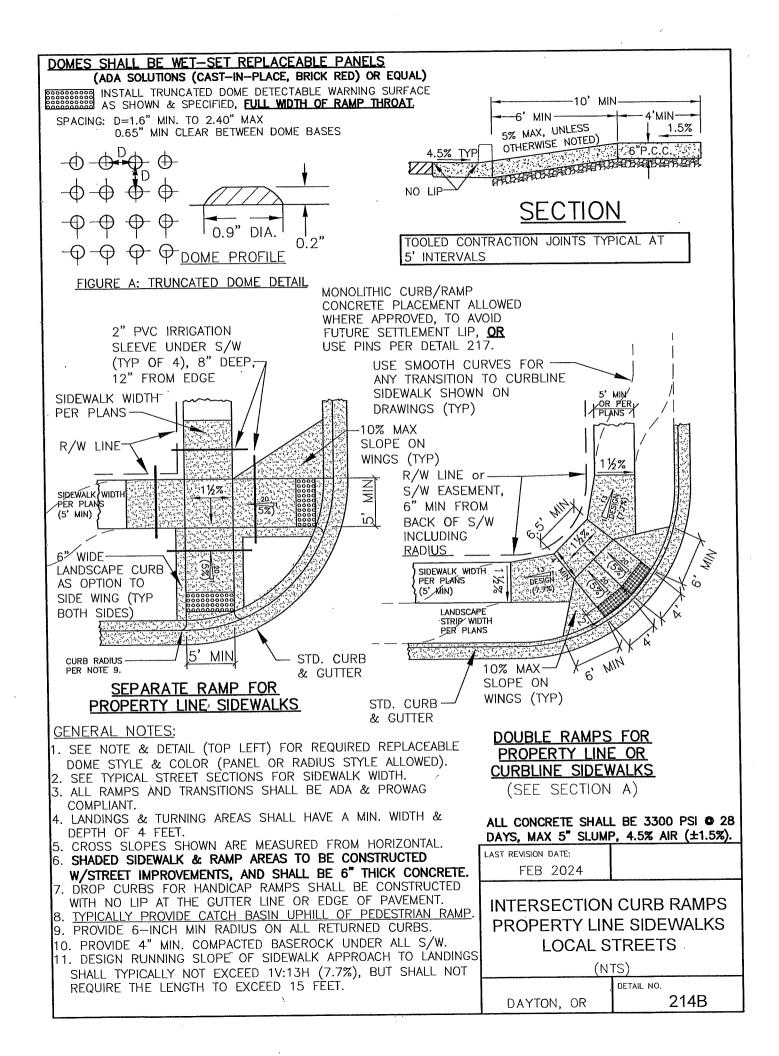
LAST REVISION DATE:	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
JUNE 2019	
48" SQUARE TREE WELL COVER PANELS (NTS)	
DAYTON, OR	DETAIL NO. 213B1

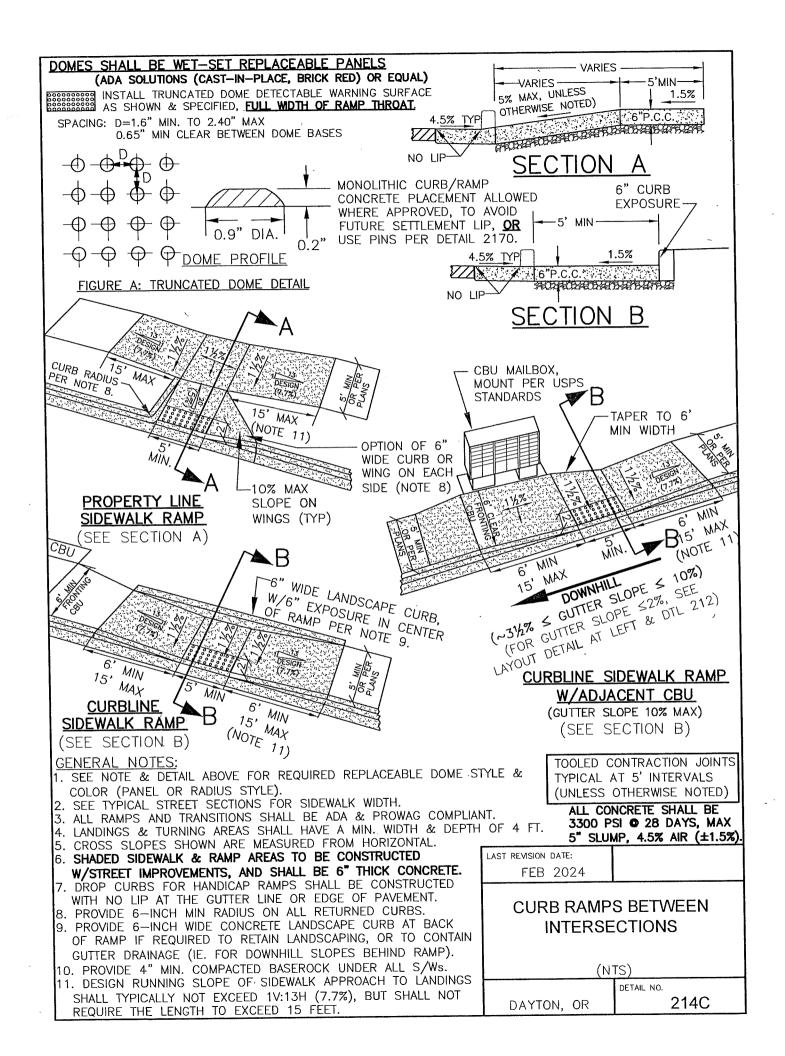


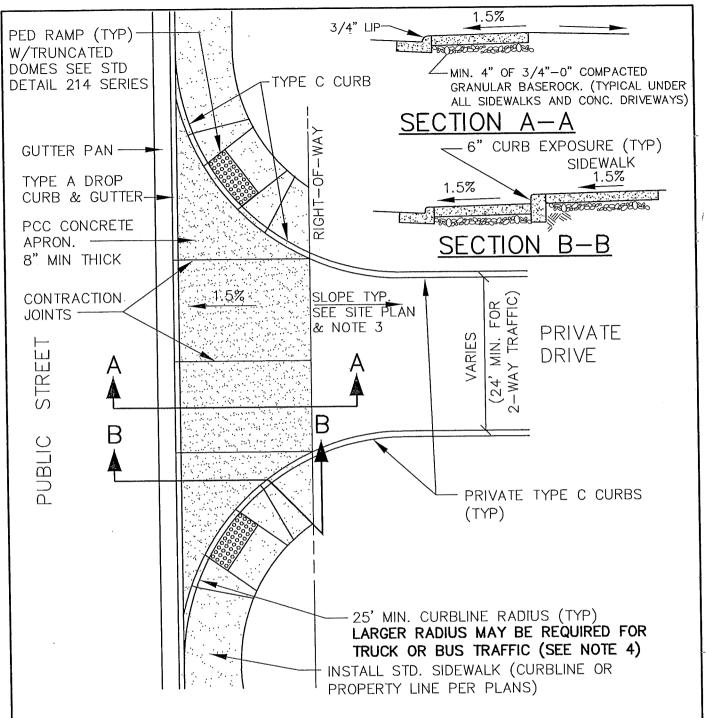












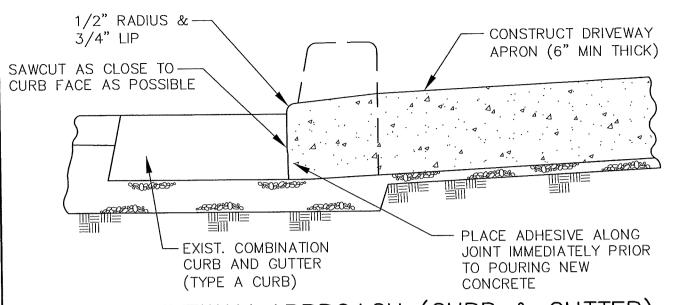
- 1. WHERE APPROVED BY THE CITY ENGINEER & PUBLIC WORKS DIRECTOR, A "DUSTPAN" STYLE APRON PER DETAILS 212A OR 213 MAY BE USED FOR COMMERCIAL/INDUSTRIAL DRIVEWAYS (BASED ON CONCRETE THICKNESS/REINFORCING AS NOTED HEREIN).
- 2. DRIVEWAY APRON SHALL BE 8" MIN THICKNESS CONCRETE.
- 3. PRIVATE CATCH BASINS ARE REQUIRED BEHIND DRIVEWAY APRON IF THE DRIVEWAY OR THE PARKING LOT BEYOND DRIVEWAY APRON SLOPES & DRAINS TOWARD THE STREET (OR ACROSS A PEDESTRIAN PATH).
- 4. TURNING RADIUS OF ANTICIPATED LARGEST VEHICLE TO BE VERIFIED DURING DESIGN.
- 5. MONOLITHIC CURB & DRIVEWAY APRON PLACEMENT IS NOT PERMITTED (IE. CURB CONCRETE & DRIVEWAY APRON CONCRETE SHALL BE PLACED SEPARATELY).
- 6. ALL CONCRETE SHALL BE 3300 PSI @ 28 DAYS, MAX 5" SLUMP, 4.5% AIR (±1.5%).

COMMERCIAL/INDUSTRIAL
DRIVEWAY APPROACH,
HIGH=VOLUME/TRUCK OPTION
(NTS)

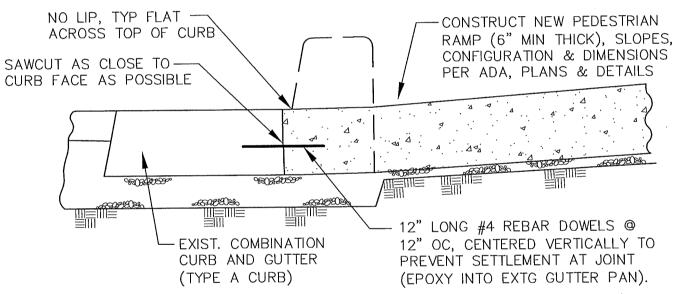
DAYTON, OR

COMMERCIAL/INDUSTRIAL
DRIVEWAY APPROACH,
HIGH=VOLUME/TRUCK OPTION
(NTS)

DETAIL NO.
216



NEW DRIVEWAY APPROACH (CURB & GUTTER)



NEW PEDESTRIAN RAMP (CURB & GUTTER)

NOTES:

- 1. ONLY ALLOWED ON EXISTING PAVED STREETS.
- 2. HORIZONTAL SAWCUTTING OF CURB TO MATCH NEW APPROACH PROFILE IS ALSO ALLOWED (SMOOTH FACED CURB GRINDING IS PROHIBITED).
- 3. SAWCUT THROUGH GUTTER PAN SHALL BE MADE AS CLOSE TO CURB FACE AS POSSIBLE.
- COMPLETE CURB AND GUTTER SHALL NOT BE REMOVED UNLESS APPROVED IN WRITING BY THE CITY ENGINEER PRIOR TO START OF CONSTRUCTION.
- 5. REPAVING IN FRONT OF FULL DEPTH CURB WHEN REMOVED. WHEN TYPE 'C' FULL DEPTH CURBS ARE REMOVED, A MIN OF 2 FEET OF PAVEMENT (MEASURED FROM THE FACE OF CURB) SHALL BE REMOVED AND REPLACED, UNLESS OTHERWISE APPROVED IN WRITING BY THE CITY ENGINEER.
- BENCH GRINDING. ANY AC SAWCUTS WILL REQUIRE A BENCH GRIND (PER DETAILS 219 & 302A) IN CONJUNCTION WITH REPAVING.
- 7. ALL CONCRETE SHALL BE 3300 PSI @ 28 DAYS, MAX 5" SLUMP, 4.5% AIR $(\pm 1.5\%)$.

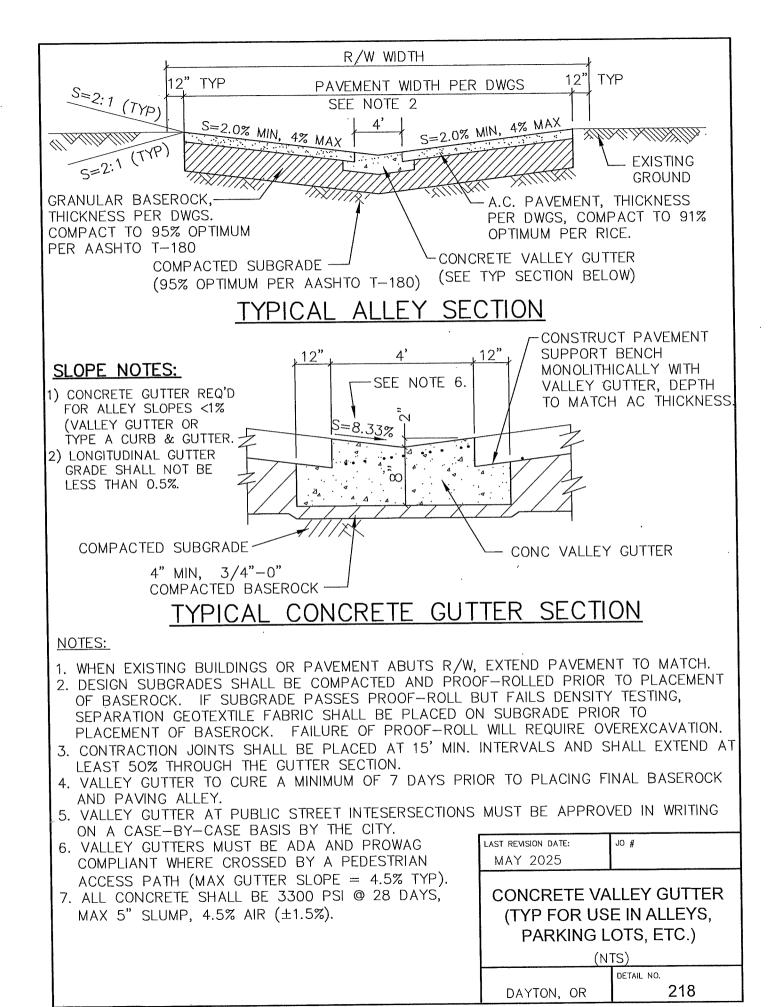
LAST REVISION DATE: FEB '2024

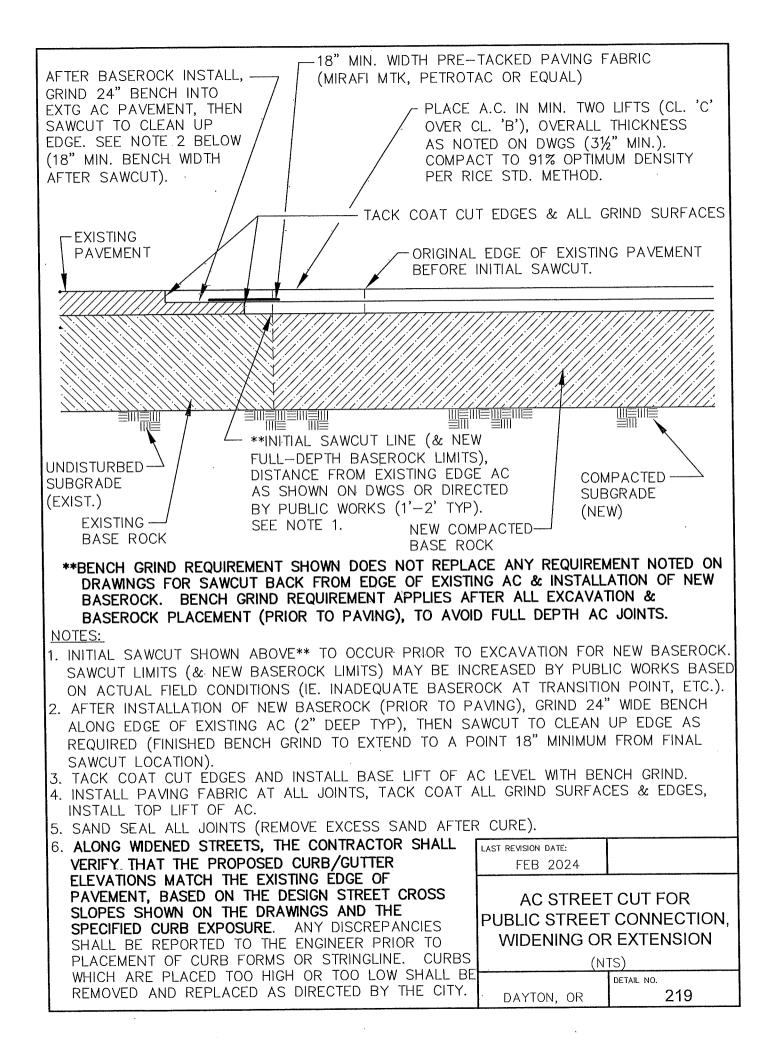
CURB CUT FOR NEW DRIVEWAYS OR PEDESTRIAN RAMP ON EXISTING CURB

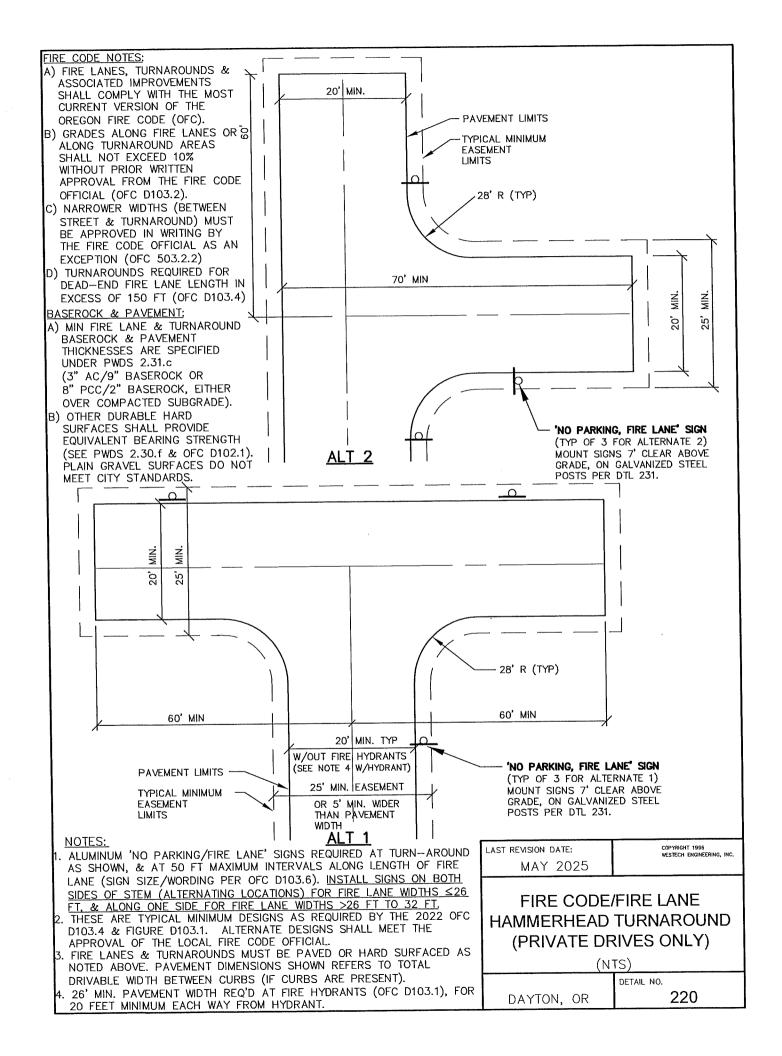
(NTS)

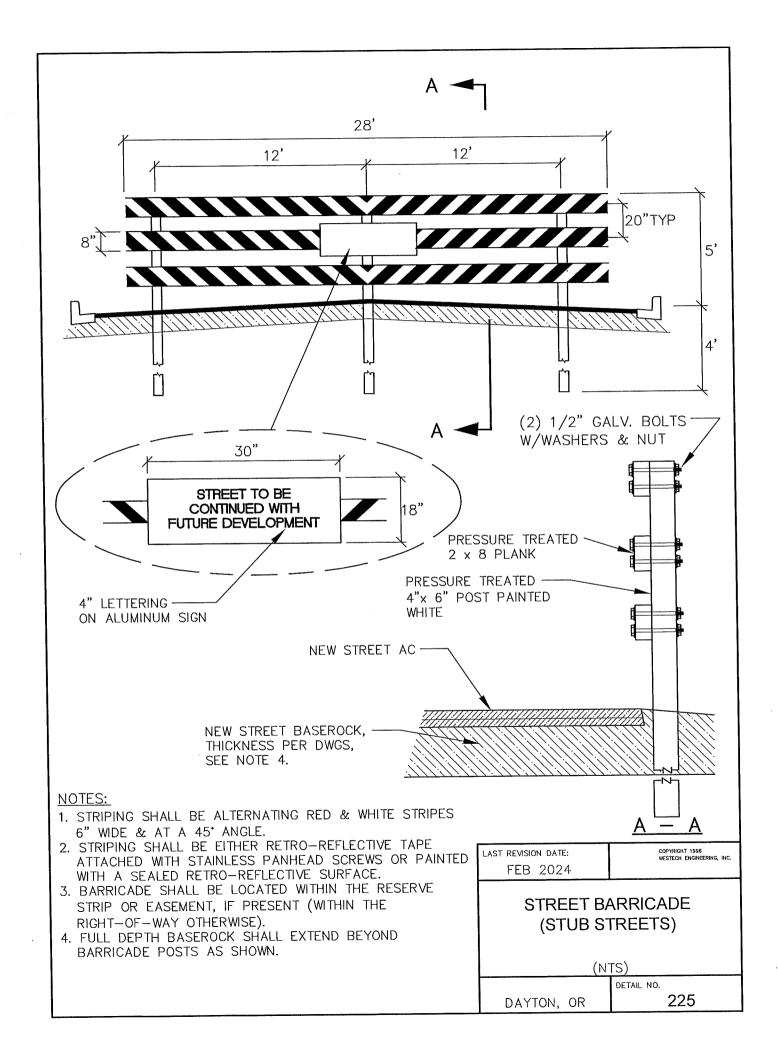
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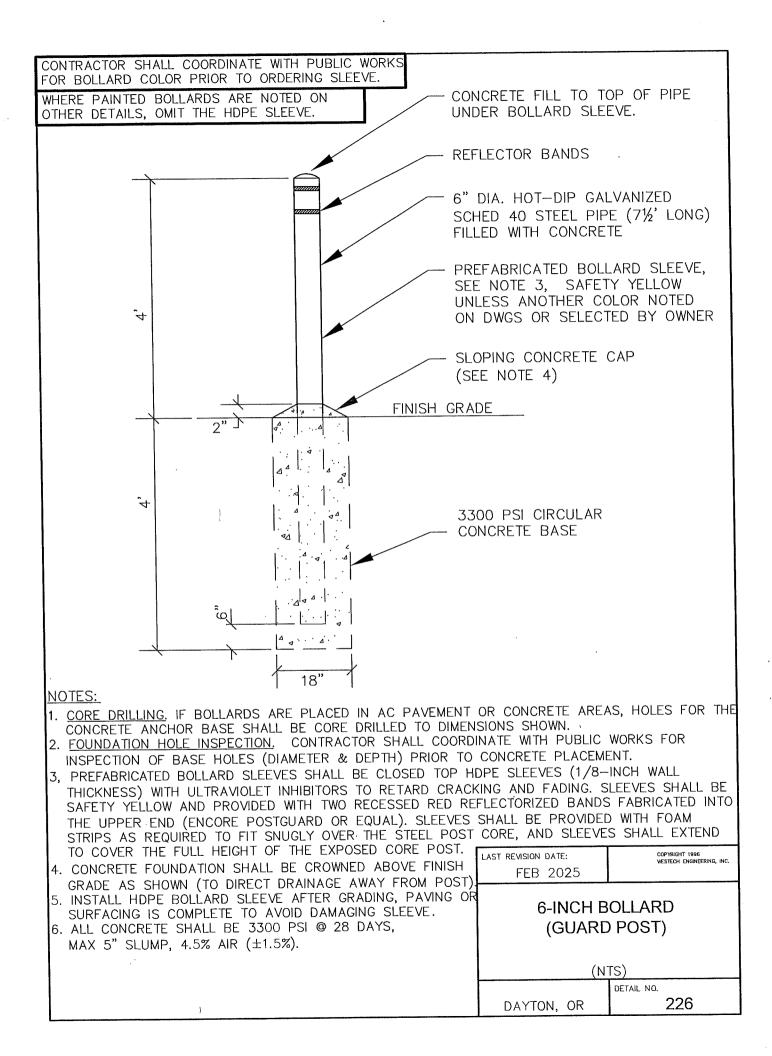
DAYTON, OR 217

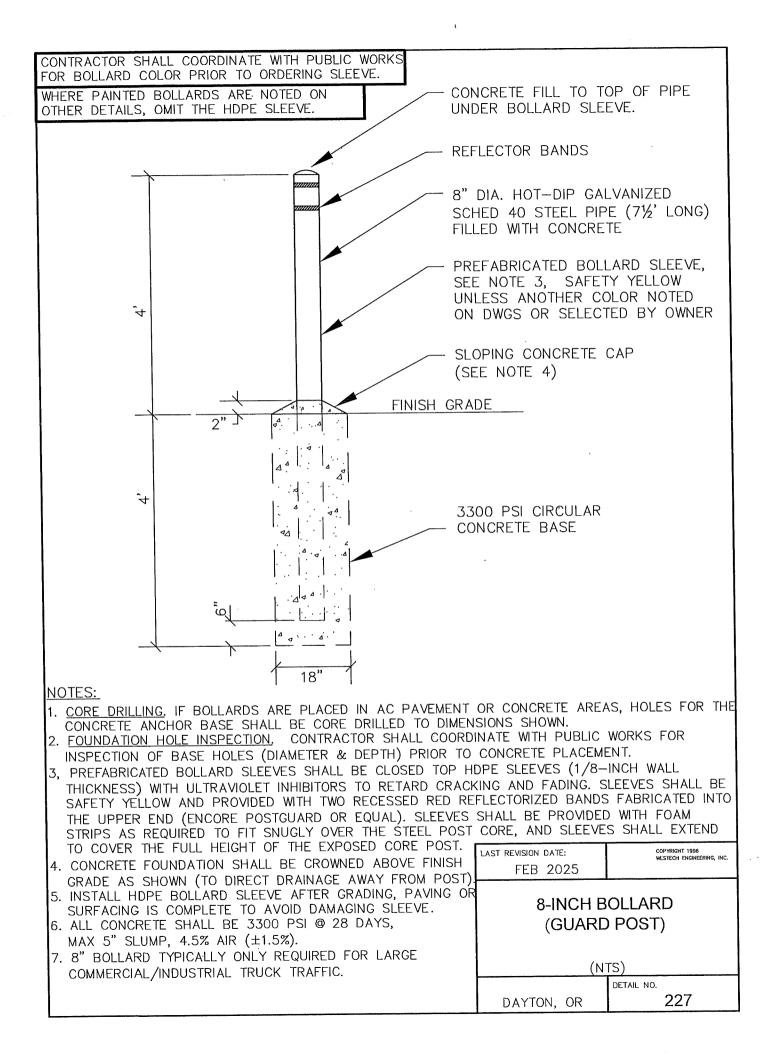


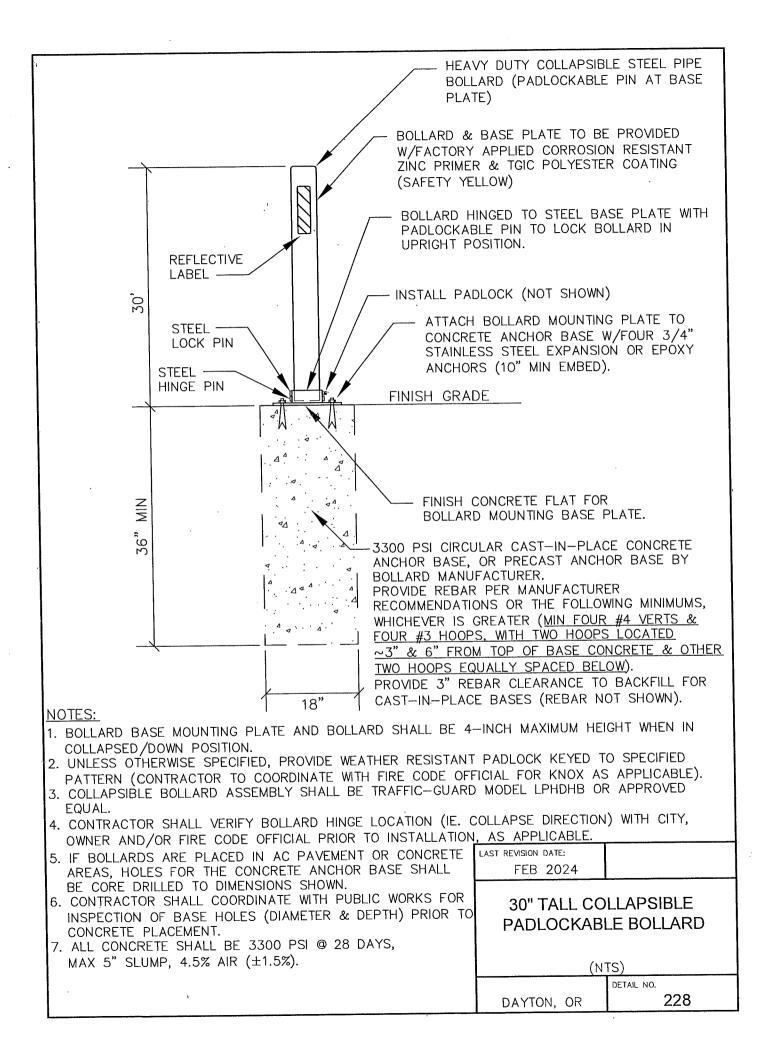


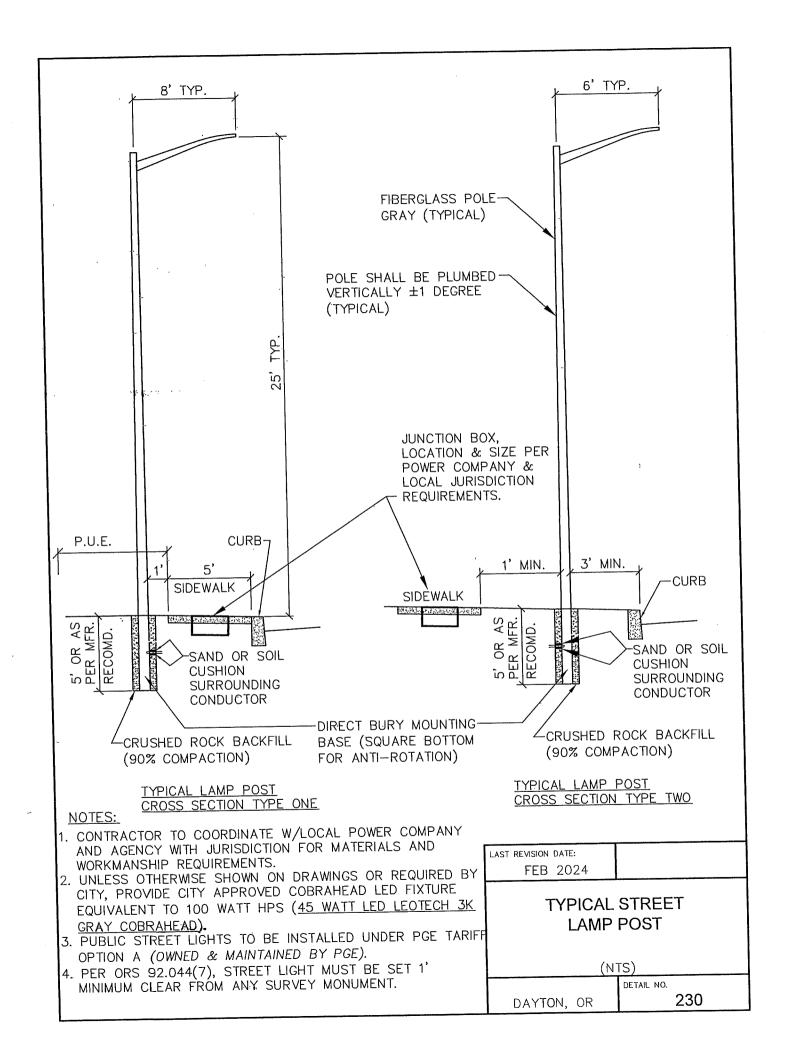


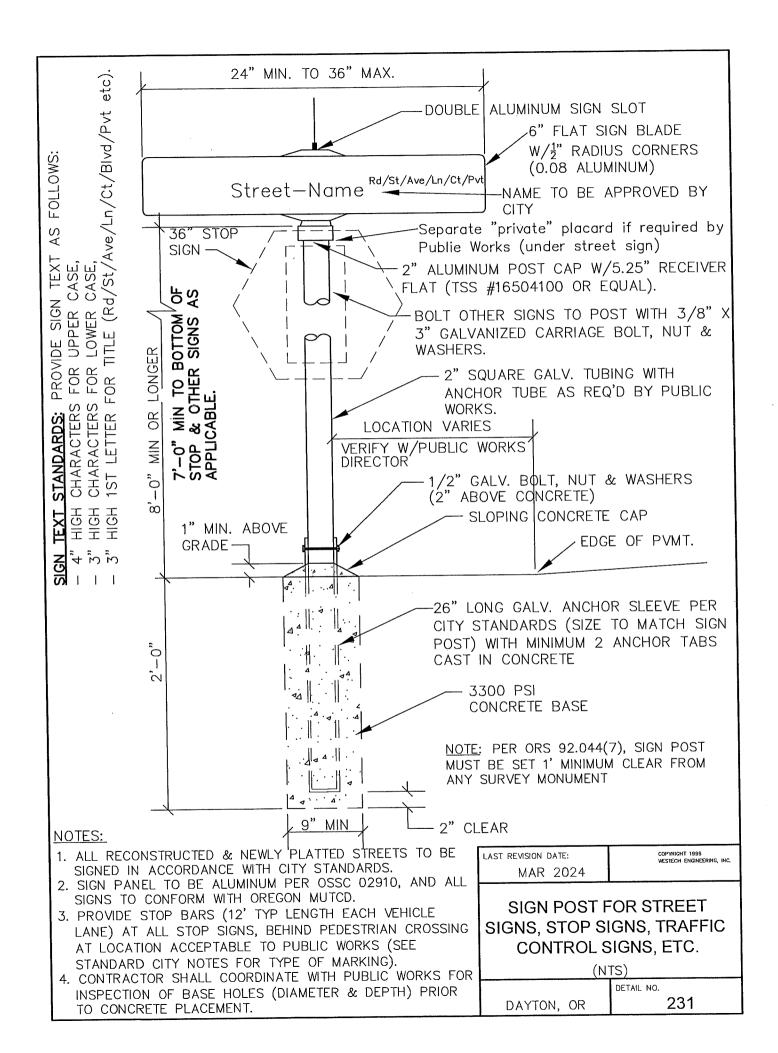


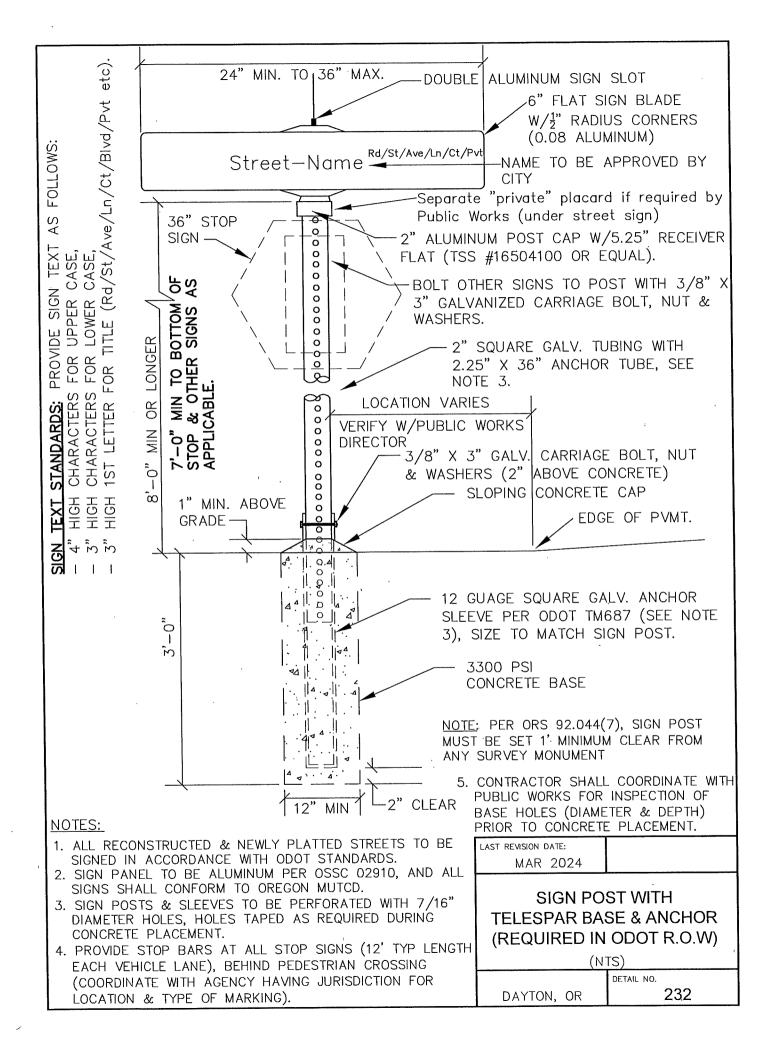








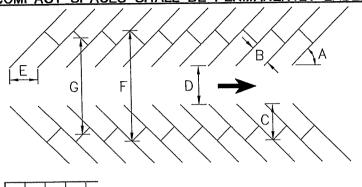




OFF-STREET PARKING DIMENSIONS

THE DISTRIBUTION OF STANDARD VERSUS COMPACT STALLS WITHIN EACH PARKING LOT/PARKING FACILITY SHALL BE CLEARLY SHOWN & NOTED ON THE DRAWINGS. DISTRIBUTION TO BE 40% MAXIMUM COMPACT SPACES.

ALL COMPACT SPACES SHALL BE PERMANENTLY LABELED (PAVEMENT MARKING OR SIGNS).



BACKING-POCKET FOR HEAD-IN PARKING WITHOUT DRIVE AISLE EXIT (MIN BACKING-POCKET WIDTH IS SAME AS WIDTH FOR STANDARD PARKING STALL).

- A- PARKING ANGLE
- B- STALL WIDTH
- C- STALL TO CURB DEPTH
- D- DRIVE AISLE WIDTH BETWEEN STALL LINES (SEE NOTE 1&2)
- E- STALL WIDTH PARALLEL TO AISLE
- F- MODULE WIDTH (FRONT OF STALL)
- G- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL AT BUMPER MIDPOINT

OFF-STREET PARKING MATRIX

MINIMUM PARKING SPACE AND AISLE DIMENSIONS (FT)
ONE WAY TRAFFIC FLOW

COMPACT (8.5' x 16')							STANDARD (9' x 19')					
A	В	С	D	E	F	G	В	С	D	E	F	G
0.	8.0	8.0	12.0	19.0	28.0		8.0	8.0	12.0	22.0	28.0	
30.	8.5	15.4	12.0	17.0	41.7	34.4	9.0	17.3	12.0	18.0	45.6	37.8
45*	8.5	17.3	13.0	12.0	47.6	41.6	9.0	19.8	13.0	12.7	52.6	46.2
60.	8.5	18.1	18.0	9.8	54.2	50.0	9.0	21.0	18.0	10.4	60.0	55.7
70	8.5	17.9	19,0	9.0	54.9	52.0	9.0	21.0	19.0	9.6	61.0	57.8
90,	8.5	16.0	24.0	8.5	56.0	56.0	9.0	19.0	24.0	9.0	62.0	62.0

NOTES:

1. WHERE PARKING LOT DRIVE AISLE IS A FIRE LANE, WIDTHS SHALL CONFORM WITH THE OREGON FIRE CODE (OFC) MINIMUMS OF 20 FEET IN ALL CASES (26 FOOT MINIMUM WIDTH, 20 FEET EACH WAY FROM FIRE HYDRANTS), PER OFC 503.2.1 & D103.1.

2. DRIVE AISLE WIDTH "D" IS REQUIRED FOR DRIVING / BACKING / TURNING MOVEMENTS ON BOTH SINGLE LOADED AND DOUBLE LOADED DRIVE AISLES.

3. SEE PWDS 2.28.J FOR ALLOWABLE <u>STANDARD</u>
PARKING SPACE LENGTH REDUCTION WITH SIDEWALKS
6' OR WIDER TO ACCOMODATE BUMPER OVERHANG.
<u>LENGTH OF COMPACT SPACES NOT TO BE REDUCED</u>.

4. NUMBER & LOCATION OF ACCESSIBLE PARKING SPACES FOR EACH PARKING LOT/PARKING FACILITY SHALL BE PROVIDED PER OSSC 1106.

LAST REVISION DATE:
OCT 2024

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OFFSTREET PARKING
DIMENSIONS
ONE WAY TRAFFIC FLOW

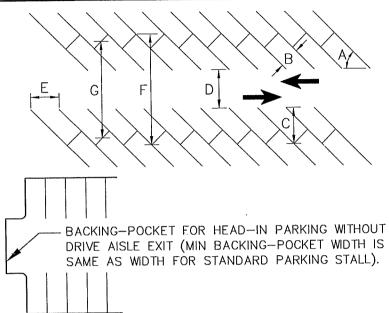
(NTS)

DAYTON, OR 235

OFF-STREET PARKING DIMENSIONS

THE DISTRIBUTION OF STANDARD VERSUS COMPACT STALLS WITHIN EACH PARKING LOT/PARKING FACILITY SHALL BE CLEARLY SHOWN & NOTED ON THE DRAWINGS. DISTRIBUTION TO BE 40% MAXIMUM COMPACT SPACES.

ALL COMPACT SPACES SHALL BE PERMANENTLY LABELED (PAVEMENT MARKING OR SIGNS).



- A- PARKING ANGLE
- B- STALL WIDTH
- C- STALL TO CURB DEPTH
- D- DRIVE AISLE WIDTH BETWEEN STALL LINES (SEE NOTE 1&2)
- E- STALL WIDTH PARALLEL TO AISLE
- F- MODULE WIDTH (FRONT OF STALL)
- G- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL AT BUMPER MIDPOINT

OFF-STREET PARKING MATRIX

MINIMUM PARKING SPACE AND AISLE DIMENSIONS (FT)
TWO WAY TRAFFIC FLOW

i												
COMPACT (8.5' x 16')							STANDARD (9' x 19')					
Α	В	С	D	E	F	G	В	С	D	E	F	G
. 0.	8.0	8.0	24.0	19.0	40.0	_	8.0	8.0	24.0	22.0	40.0	
30°	8.5	15.4	24.0	17.0	54.8	47.4	9.0	17.3	24.0	18.0	58.6	50.8
45°	8.5	17.3	24.0	12.0	58.6	52.9	9.0	19.8	24.0	12.7	63.6	57.2
60°	8.5	18.1	24.0	9.8	60.2	56.0	9.0	21.0	24.0	10.4	66	61.5
70'	8.5	17.9	24.0	9.0	59.8	56.9	9.0	21.0	24.0	9.6	66	62.9
90.	8.5	16.0	24.0	8.5	56.0	56.0	9.0	19.0	24.0	9.0	62.0	62.0

NOTES:

1. WHERE PARKING LOT DRIVE AISLE IS A FIRE LANE, WIDTHS SHALL CONFORM WITH THE OREGON FIRE CODE (OFC) MINIMUMS OF 20 FEET IN ALL CASES (26 FOOT MINIMUM WIDTH, 20 FEET EACH WAY FROM FIRE HYDRANTS), PER OFC 503.2.1 & D103.1.

2. DRIVE AISLE WIDTH "D" IS REQUIRED FOR DRIVING / BACKING / TURNING MOVEMENTS ON BOTH SINGLE LOADED AND DOUBLE LOADED DRIVE AISLES.

3. SEE PWDS 2.28.J FOR ALLOWABLE <u>STANDARD</u>
PARKING SPACE LENGTH REDUCTION WITH SIDEWALKS
6' OR WIDER TO ACCOMODATE BUMPER OVERHANG.
LENGTH OF COMPACT SPACES NOT TO BE REDUCED.

4. NUMBER & LOCATION OF ACCESSIBLE PARKING SPACES FOR EACH PARKING LOT/PARKING FACILITY SHALL BE PROVIDED PER OSSC 1106.

LAST REVISION DATE:

APR 2025

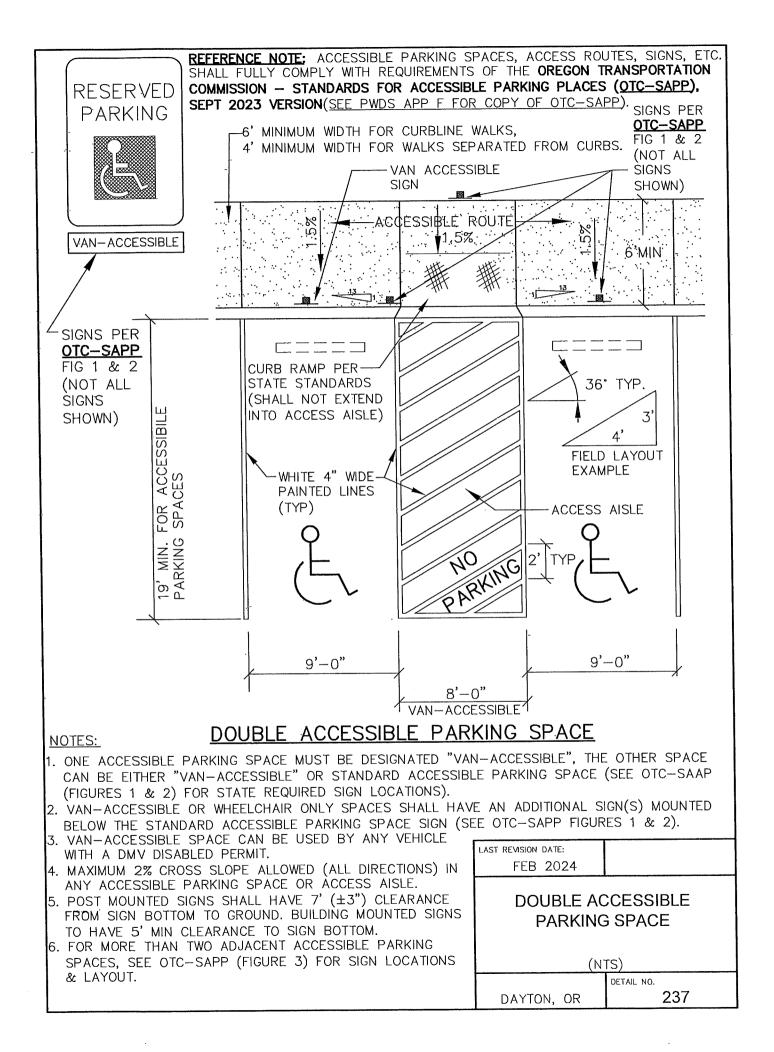
COPYRIGHT 1996 WESTECH ENGINEERING, INC.

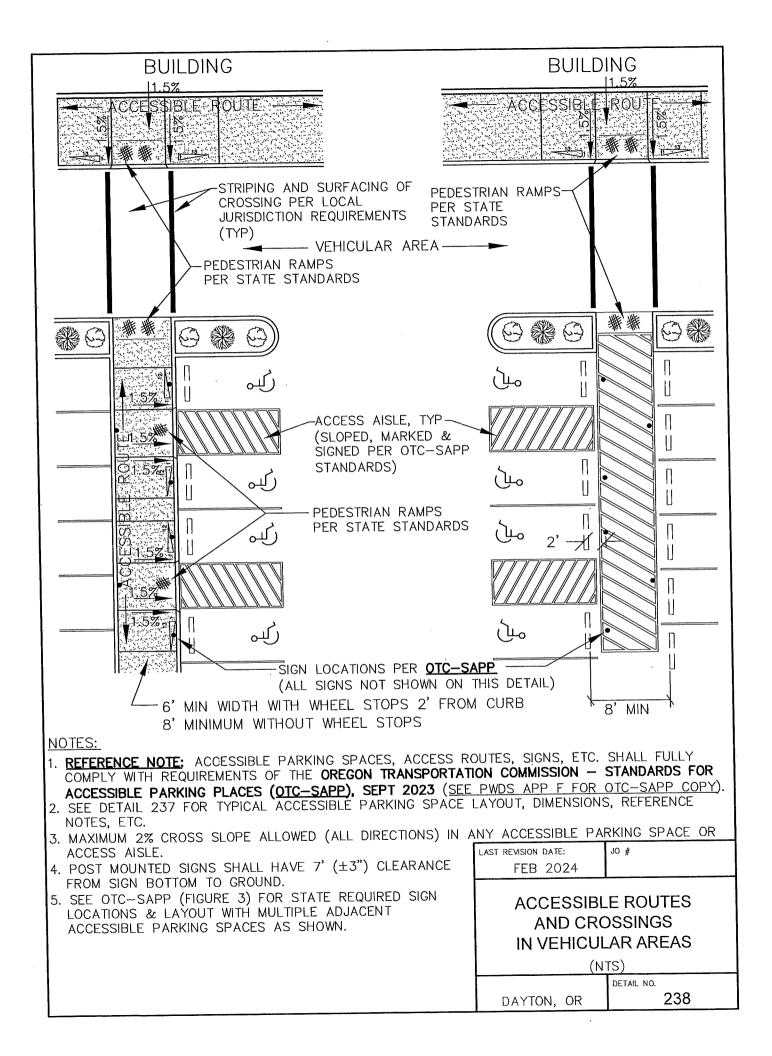
OFFSTREET PARKING
DIMENSIONS
TWO WAY TRAFFIC FLOW

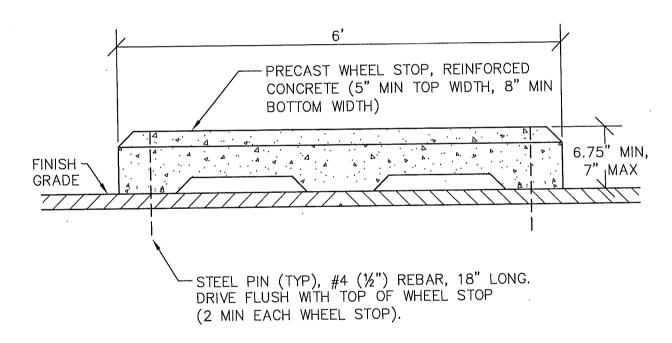
(NTS)

DAYTON, OR

236







SECTION

NOTES:

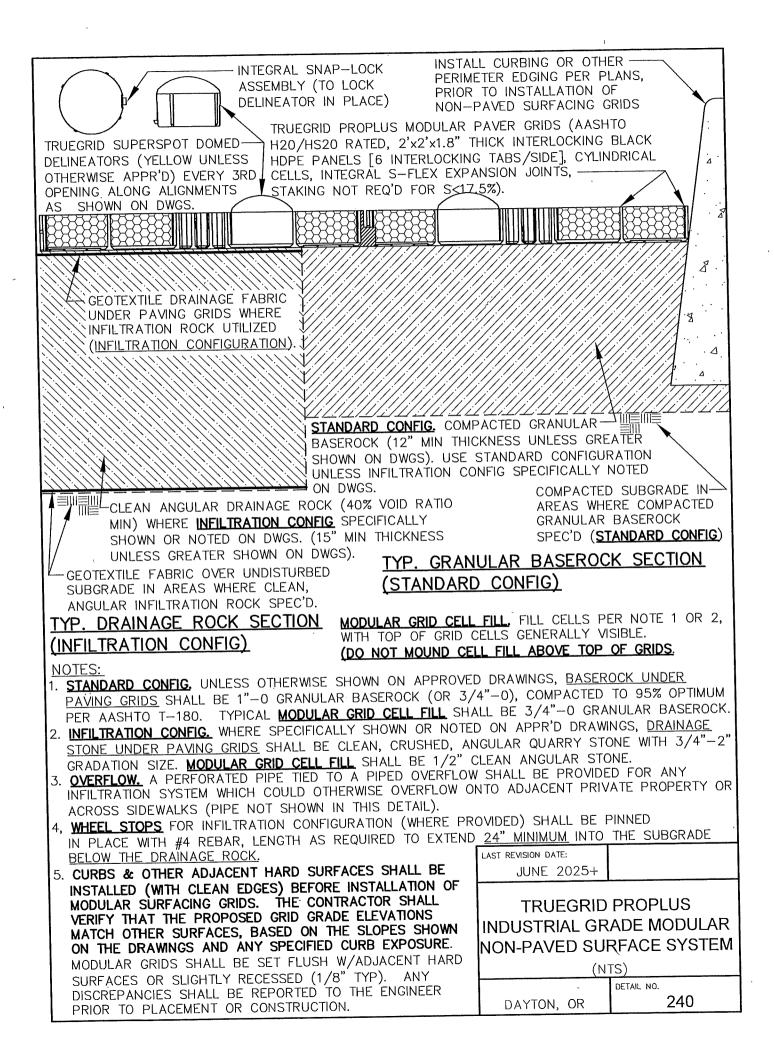
- 1. SEE DRAWINGS FOR LOCATION & NUMBER OF WHEEL STOPS, INCLUDING DIMENSION FROM CURB, EDGE OF PAVEMENT OR BUILDING AS APPLICABLE.
- 2. UNLESS OTHERWISE SPECIFIED OR SHOWN ON SITE PLAN, SET WHEEL STOPS 2 FEET FROM FACE OF CURB OR EDGE OF PAVEMENT, MEASURED FROM THE FACE OF THE WHEEL STOP (VEHICLE SIDE) TO FACE OF CURB (OR EDGE OF PAVEMENT). SET BACK FROM PROPERTY LINES PER CITY STANDARDS (3' MIN). MIN SETBACK FROM BUILDINGS AS SHOWN ON DWGS.
- 3. FOR USE ON HEAD—IN PARKING WITHOUT FULL HEIGHT CURBS, OR WHERE A SIDEWALK ALONG HEAD—IN PARKING IS LESS THAN 6 FEET WIDE.

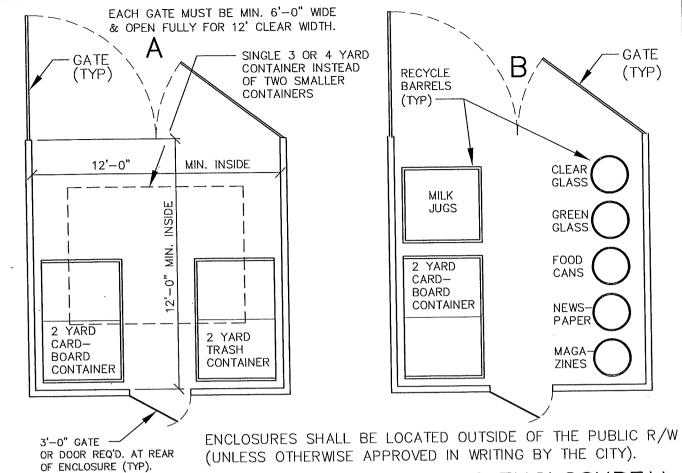
PRECAST WHEELSTOP
DETAIL

(NTS)

DAYTON, OR

239





TRASH ENCLOSURE**

RECYCLE ENCLOSURE**

**ENCLOSURES SHOWN ARE TYPICAL EXAMPLES UNLESS ALTERNATE CONFIGURATION IS APPROVED BY TRASH/RECYCLING FRANCHISEE AND THE CITY PLANNER.

NOTES:

- GATES:
 - (a) ALL GATES MUST ATTACH AT THE END OF OF THE WALLS TO PROVIDE A MINIMUM OF 12' CLEAR WORKING SPACE WHEN OPEN.
 - (b) TO SERVICE THE ENCLOSURE, THE GATES MUST BE ABLE TO BE PINNED OR BLOCKED IN THE FULL OPEN POSITION.
 - (c) GATES MUST OPEN TOWARD TEH OUTSIDE OF THE ENCLOSURE.
- 2. FOR 5 OR 6 YARD CONTAINERS THE ENCLOSURE DEPTH MUST BE 15'.
- 3. WHERE REQ'D. (I.E. RESTAURANTS), GREASE BARRELS MUST BE SEPARATE FROM TRASH AND RECYCLING ENCLOSURES.
- 4. ROOFS OR OVERHANGS SHALL HAVE 15' OF OVERHEAD CLEARANCE.
- 5. IF RECYCLING IS NOT INCLUDED, AREA (A) CAN PROVIDE SERVICE FOR TRASH AND CARDBOARD FOR CONTAINER SIZES OF 1 TO 2 YARDS. IF A 3 YARD OR LARGER TRASH CONTAINER IS NEEDED, AN ADDITIONAL 12' X 12' SPACE WILL BE NECESSARY FOR CARDBOARD CONTAINER SERVICE.

6. HARD SURFACE INSIDE & IN FRONT OF ENCLOSURES. CONCRETE OR PAVEMENT PADS SHALL BE

PROVIDED INSIDE ALL ENCLOSURES, AS WELL AS IN FRONT OF ALL ENCLOSURES AS REQUIRED TO ALLOW ENCLOSURES TO BE ROLLED OUT FOR TRUCK PICKUP.

7. WALLS, GATE & DOOR MATERIALS & HEIGHT SHALL BE PER CITY STANDARDS BASED ON SCREENING REQUIREMENTS.

8. FOR REFERENCE, A 1 YARD CONTAINER WILL HOLD APPROX. THE SAME AS 6 TRASH CANS (32 GAL SIZE). USE 6 TIMES THE NUMBER OF CANS REQUIRED TO ESTIMATE THE REQUIRED CONTAINER SIZE IN CUBIC YARDS. FOR EXAMPLE, A 3 YD. CONTAINER WILL HOLD APPROX THE SAME AMOUNT AS 18 TRASH CANS (32 GAL SIZE).

LAST REVISION DATE:

APR 2025

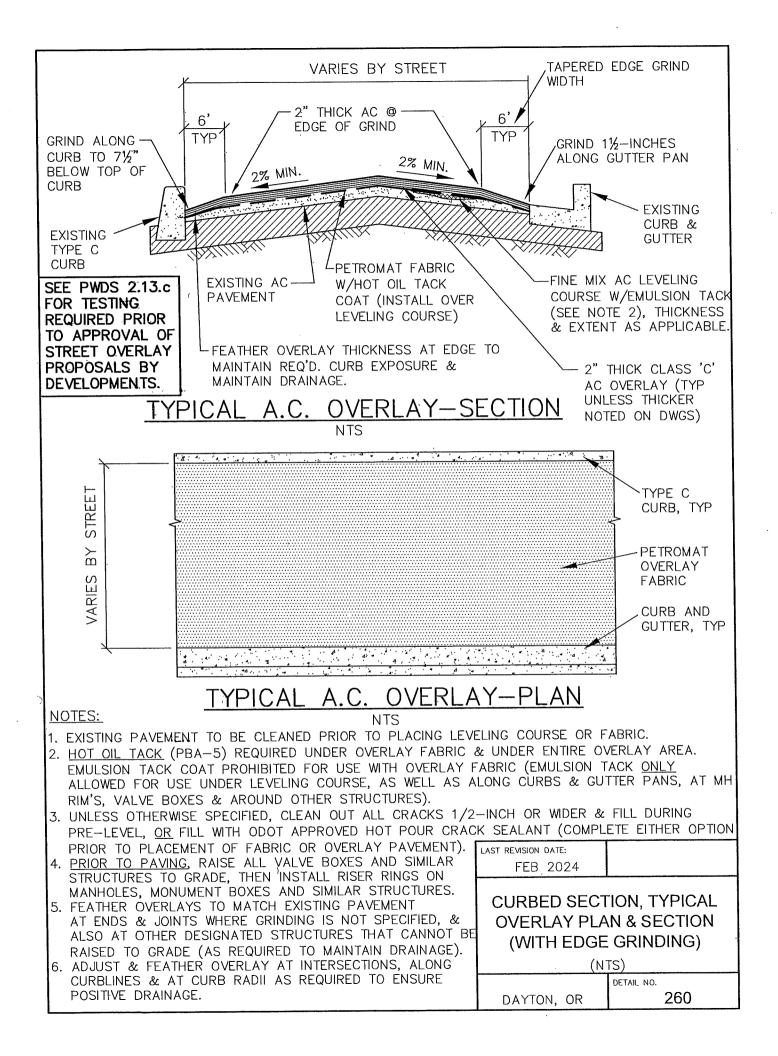
TRASH AND RECYCLING ENCLOSURE REQUIREMENTS AND EXAMPLE

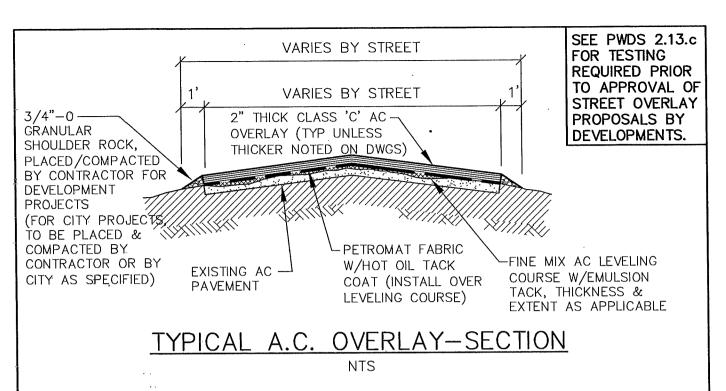
(NTS)

DAYTON, OR

DETAIL NO.

250





EXTG EDGE OF AC

OVERLAY FABRIC & AC OVERLAY

SHOULDER ROCK

TYPICAL A.C. OVERLAY-PLAN

NOTES:

NTS

- 1. EXISTING PAVEMENT TO BE CLEANED PRIOR TO PLACING LEVELING COURSE OR FABRIC.
- 2. <u>HOT OIL TACK</u> (PBA-5) REQUIRED UNDER OVERLAY FABRIC & UNDER ENTIRE OVERLAY AREA. EMULSION TACK COAT PROHIBITED FOR USE WITH OVERLAY FABRIC (EMULSION TACK <u>ONLY</u> ALLOWED FOR USE UNDER LEVELING COURSE, AS WELL AS ALONG CURBS & GUTTER PANS, AT MH RIM'S, VALVE BOXES & AROUND OTHER STRUCTURES).

3. UNLESS OTHERWISE SPECIFIED, CLEAN OUT ALL CRACKS 1/2-INCH OR WIDER & FILL DURING PRE-LEVEL, OR FILL WITH ODOT APPROVED HOT POUR CRACK SEALANT (COMPLETE EITHER OPTION PRIOR TO PLACEMENT OF FABRIC OR OVERLAY PAVEMENT). LAST REVISION DATE:

4. PRIOR TO PAVING, RAISE ALL VALVE BOXES AND SIMILAR STRUCTURES TO GRADE, THEN INSTALL RISER RINGS ON MANHOLES, MONUMENT BOXES AND SIMILAR STRUCTURES.

5. FEATHER OVERLAYS TO MATCH EXISTING PAVEMENT AT ENDS & JOINTS WHERE GRINDING IS NOT SPECIFIED, & ALSO AT OTHER DESIGNATED STRUCTURES THAT CANNOT BE RAISED TO GRADE (AS REQUIRED TO MAINTAIN DRAINAGE).

 ADJUST & FEATHER OVERLAY AT INTERSECTIONS, ALONG CURBLINES & AT CURB RADII AS REQUIRED TO ENSURE POSITIVE DRAINAGE. LAST REVISION DATE: FEB 2024

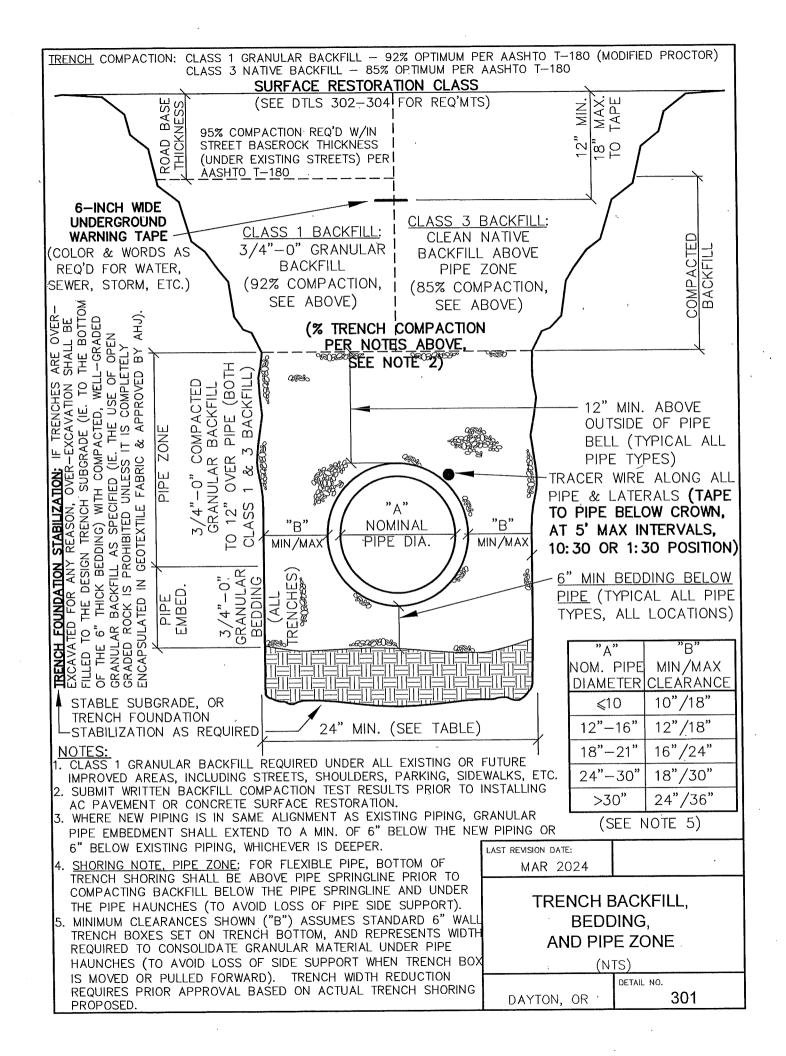
TURNPIKE SECTION, TYPICAL OVERLAY PLAN & SECTION (WITHOUT GRINDING)

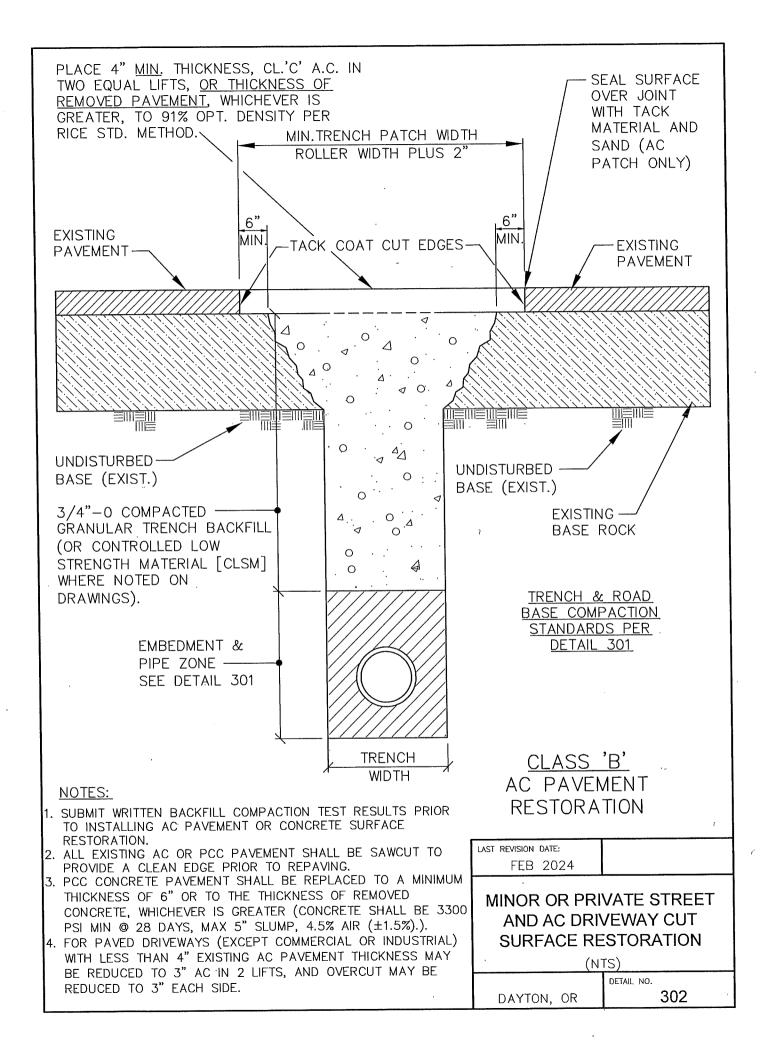
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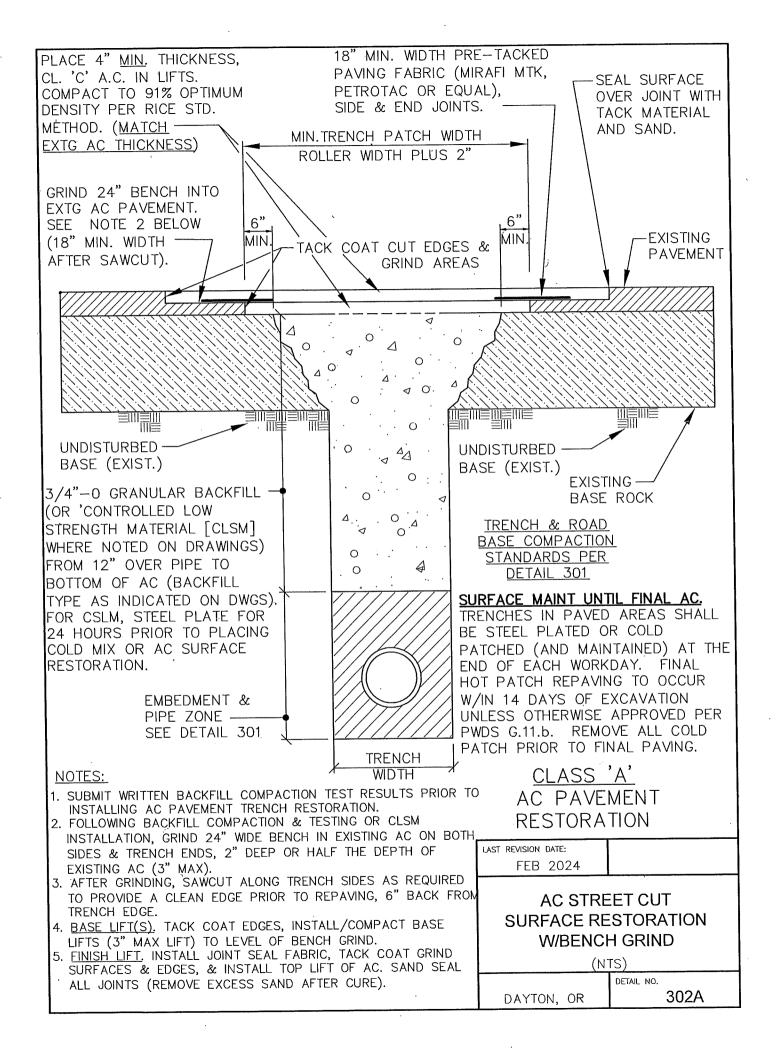
DAYTON, OR

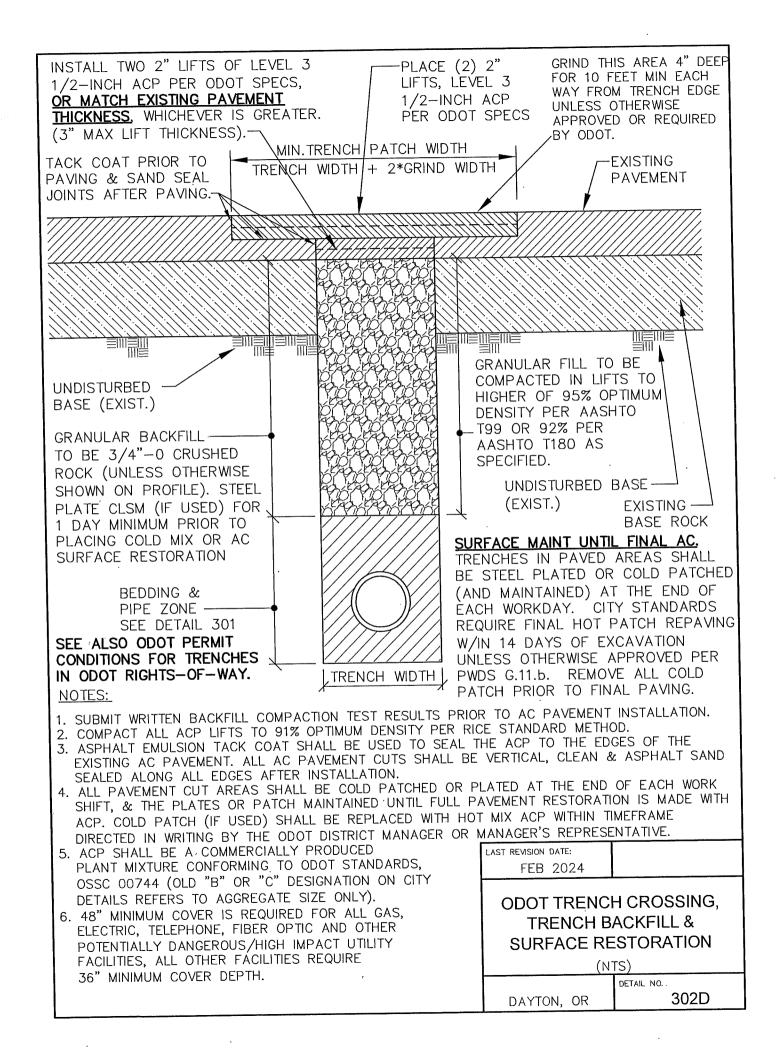
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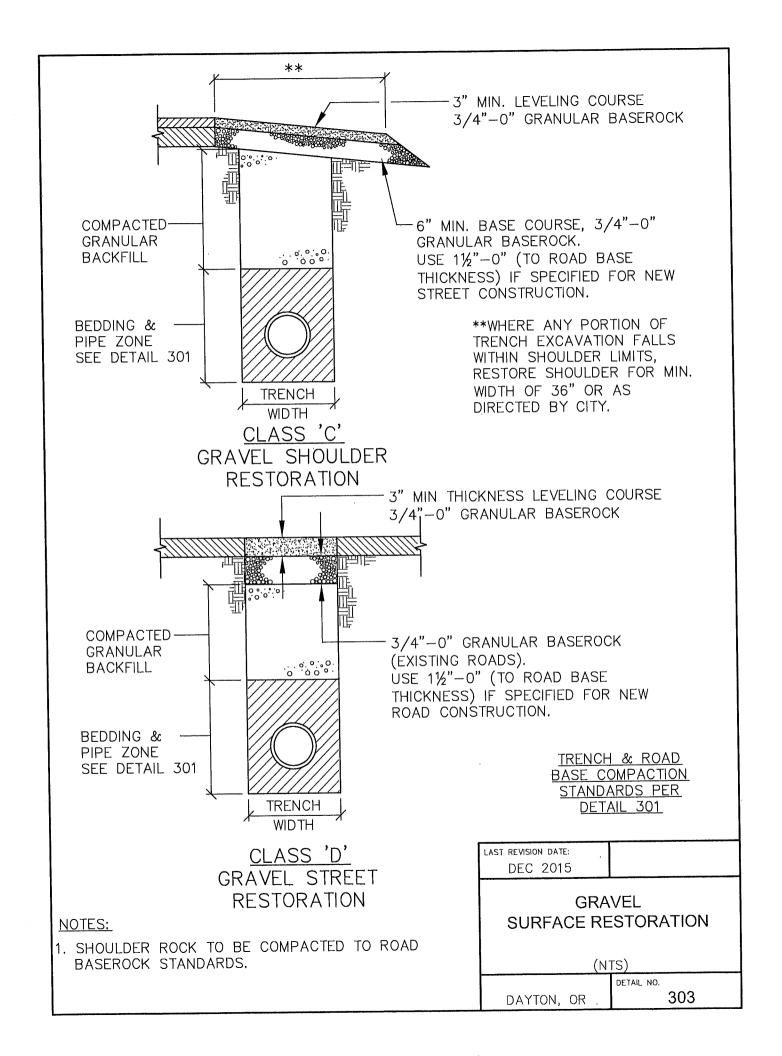
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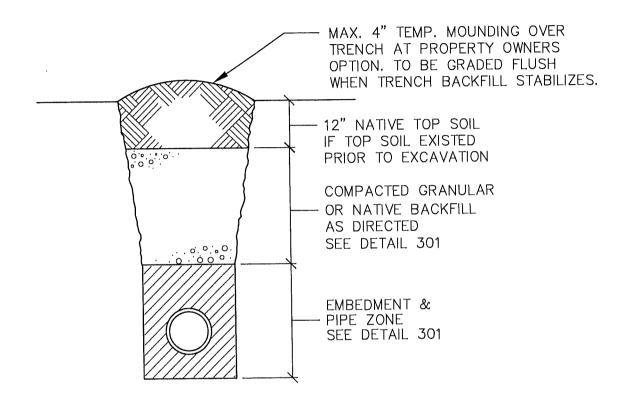












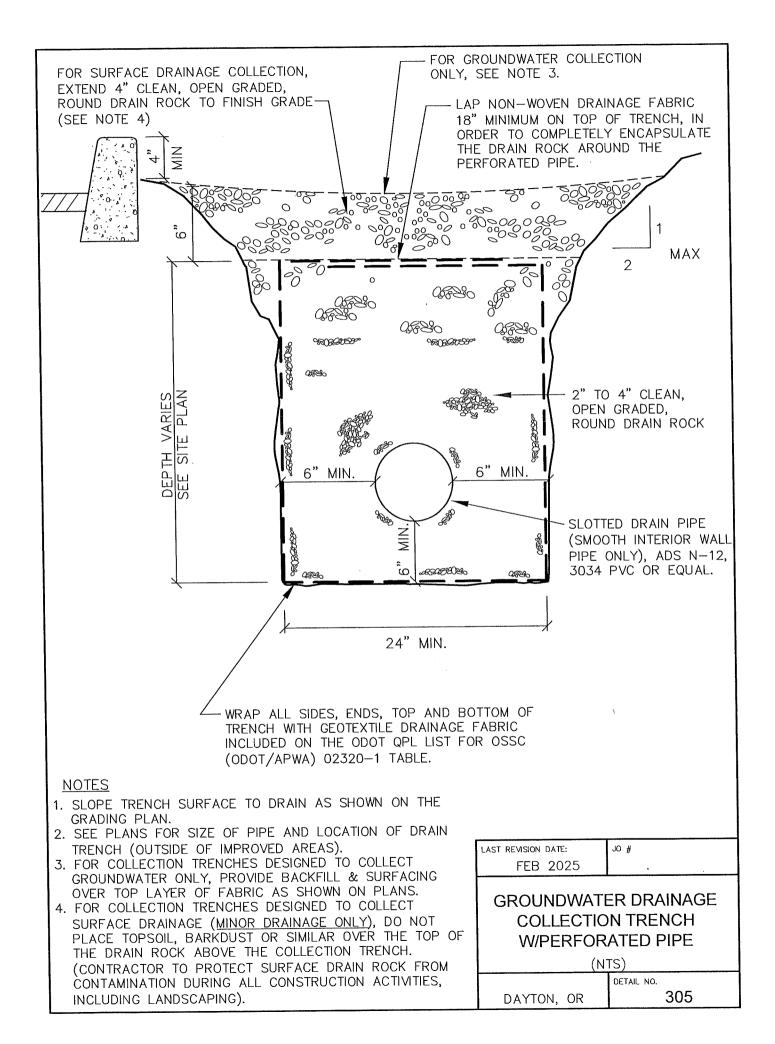
CLASS 'E'
UNIMPROVED & OPEN AREAS

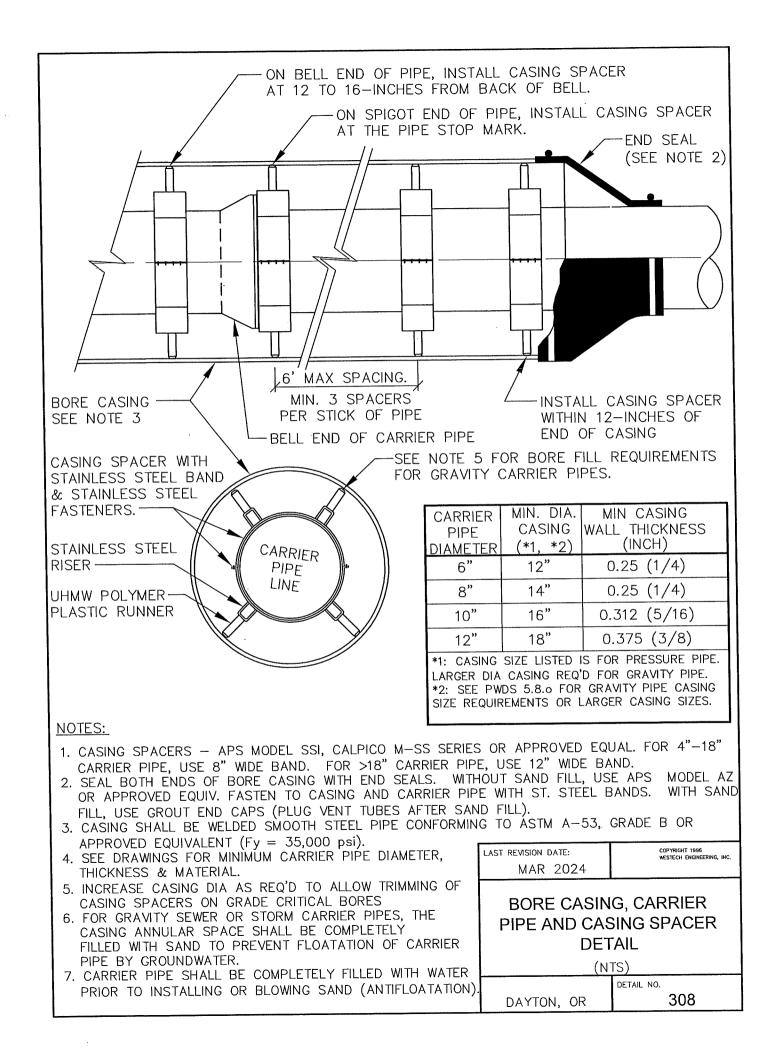
TRENCH & ROAD
BASE COMPACTION
STANDARDS PER
DETAIL 301

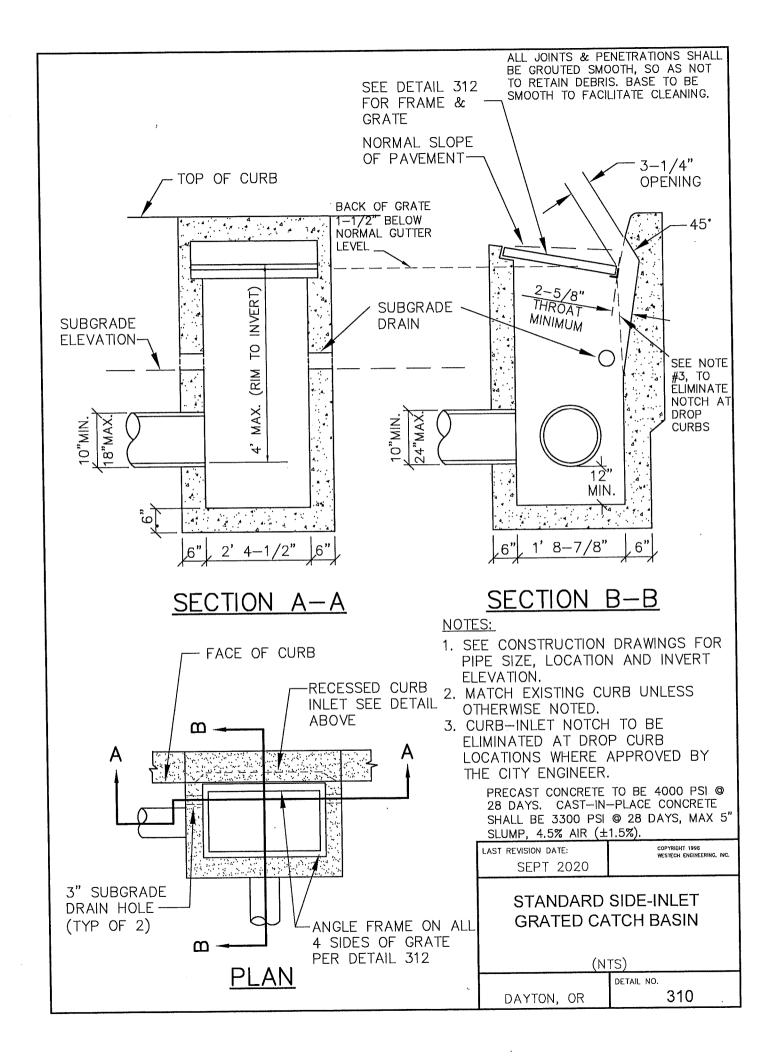
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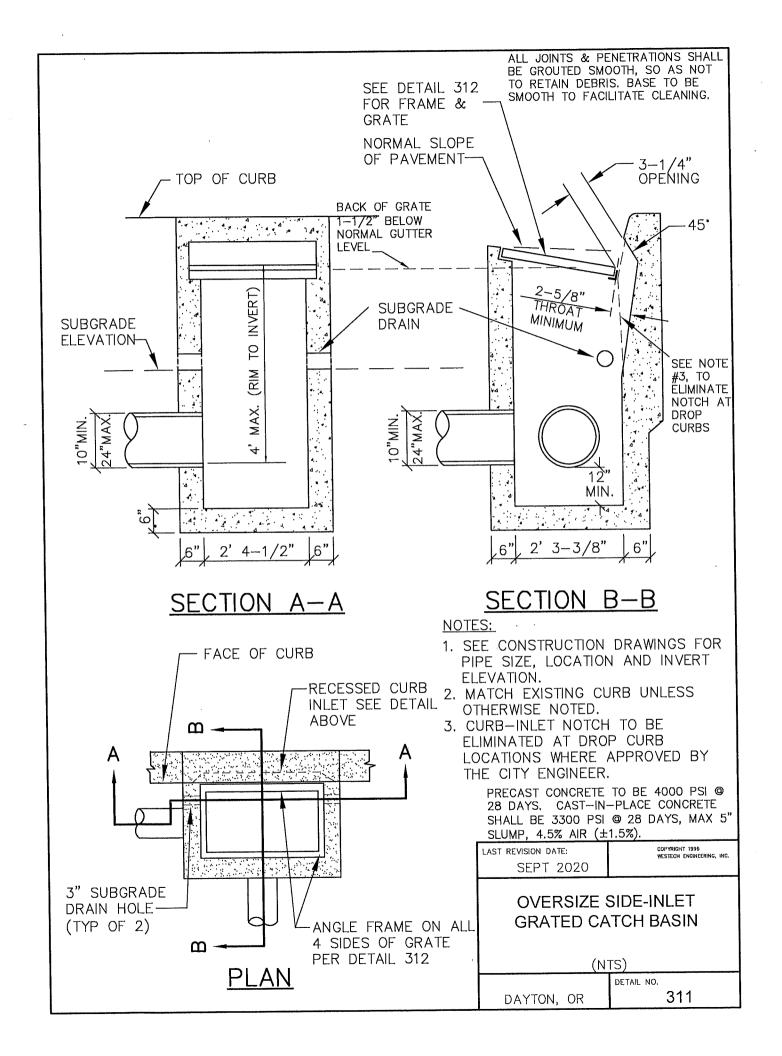
1. ANY TRENCH SETTLEMENT DURING WARRANTY PERIOD SHALL BE CORRECTED AT CONTRACTOR'S EXPENSE, INCLUDING SURFACE RESTORATION.

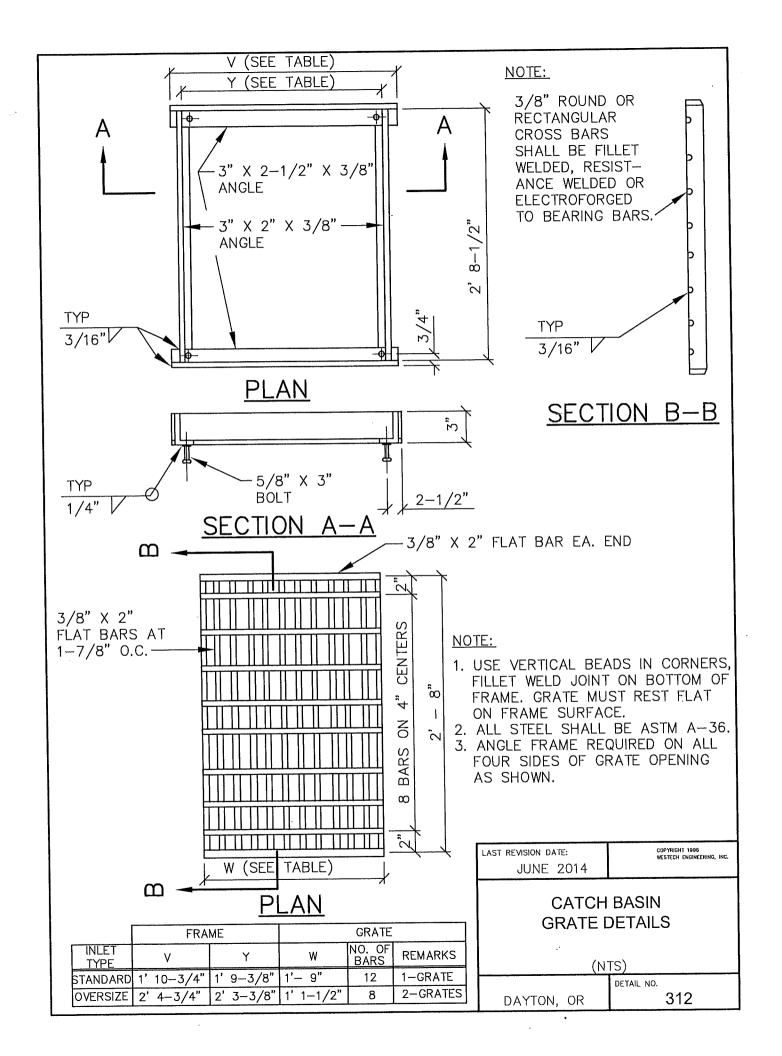
LAST REVISION DATE:							
DEC 2015							
NATIVE SURFACE RESTORATION							
(NTS)							
DAYTON, OR	detail no. 304						

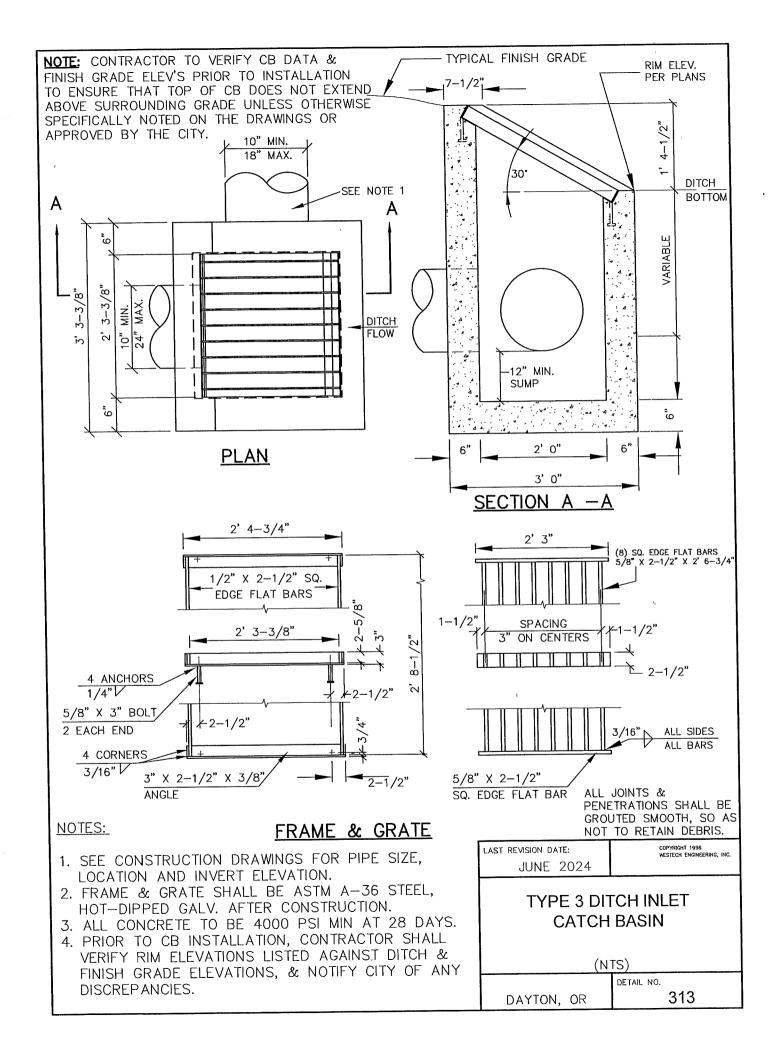


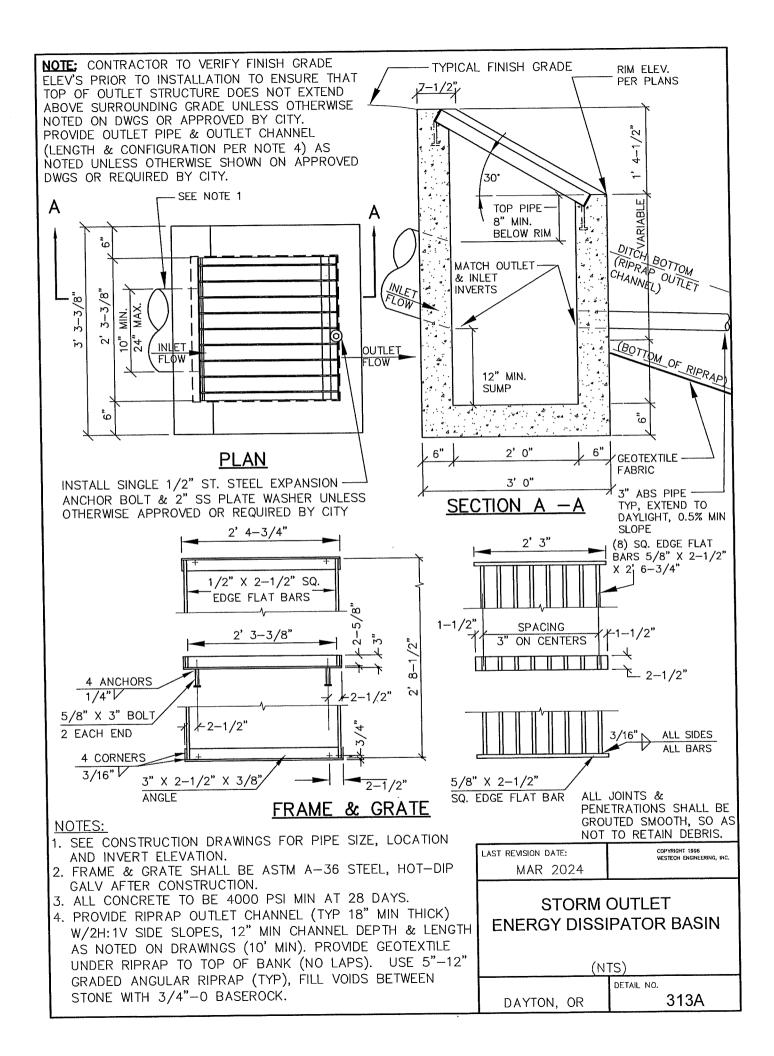


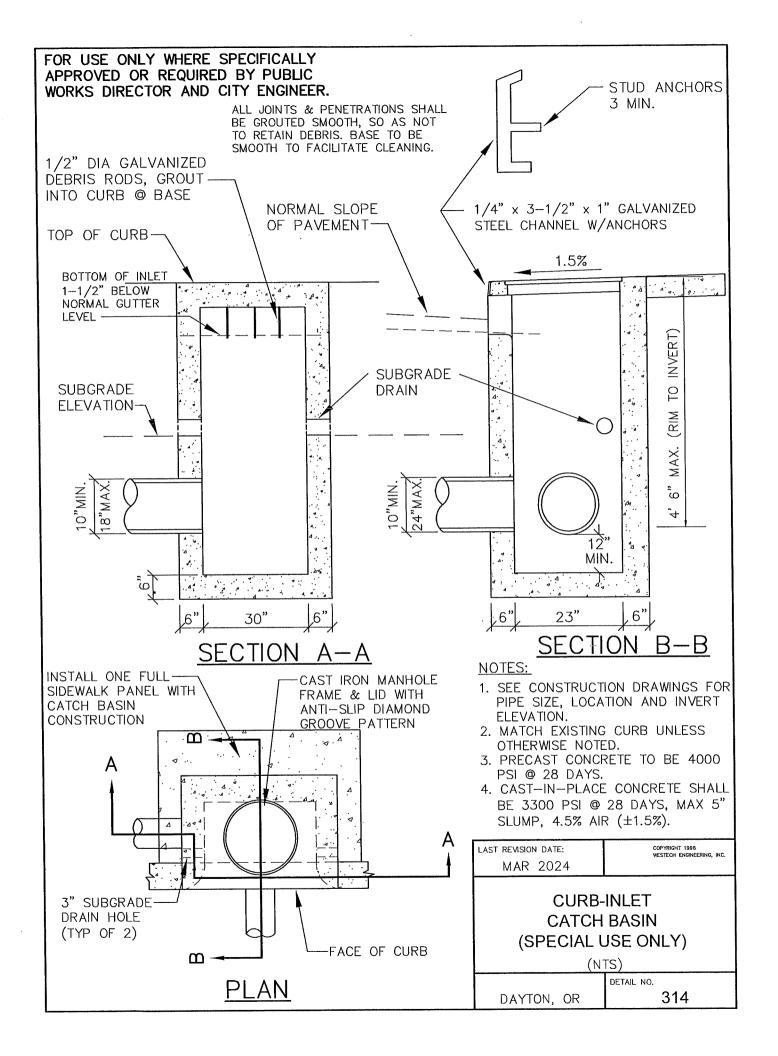


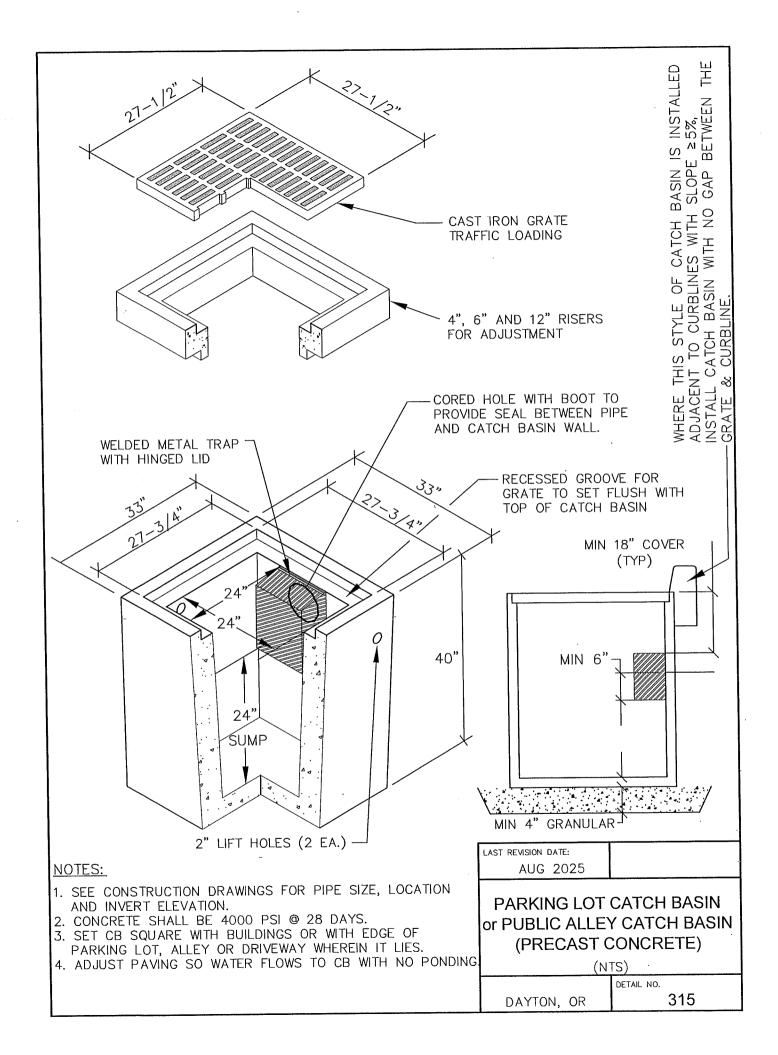


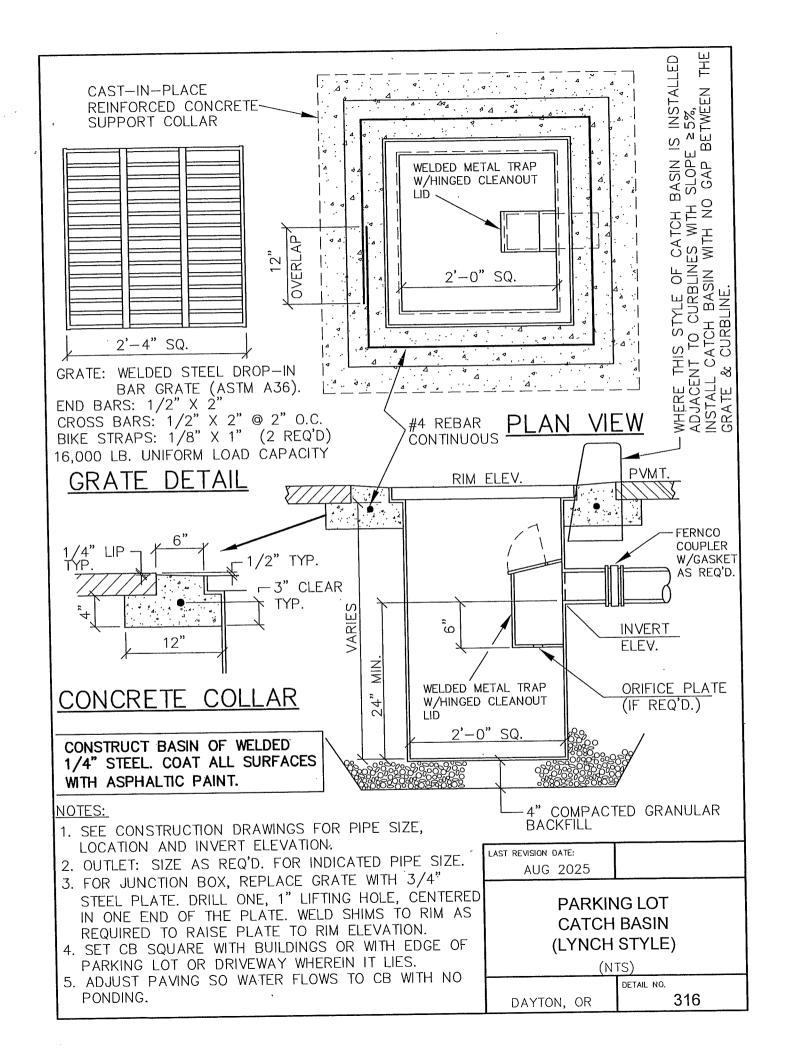


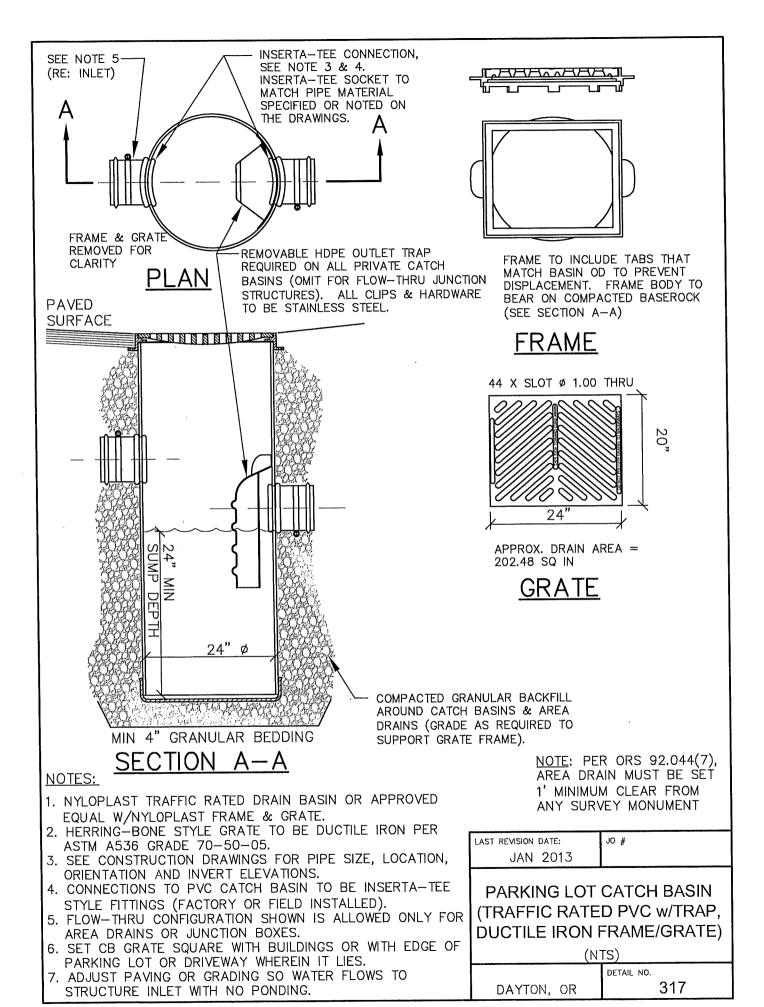


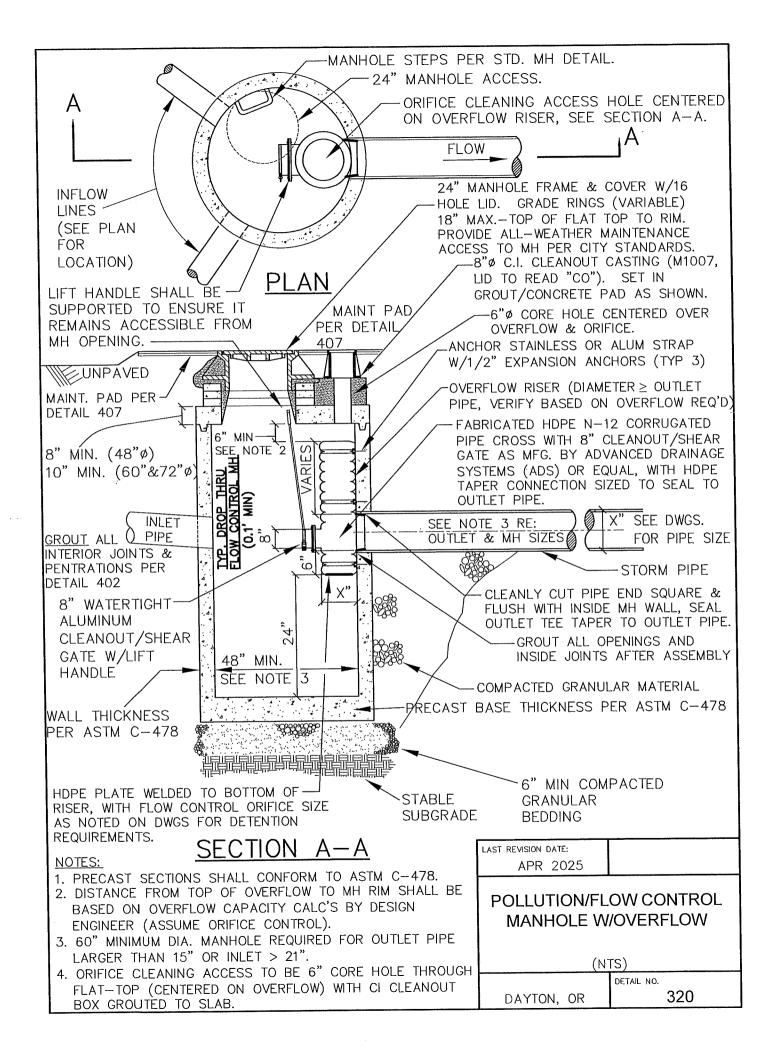


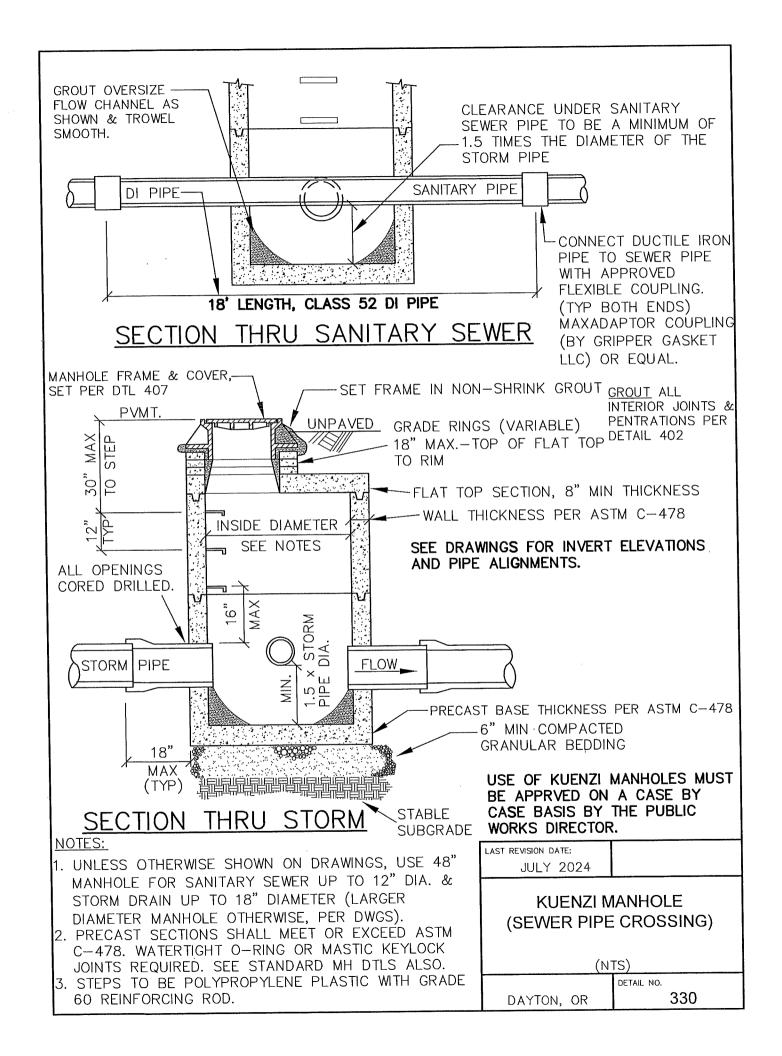


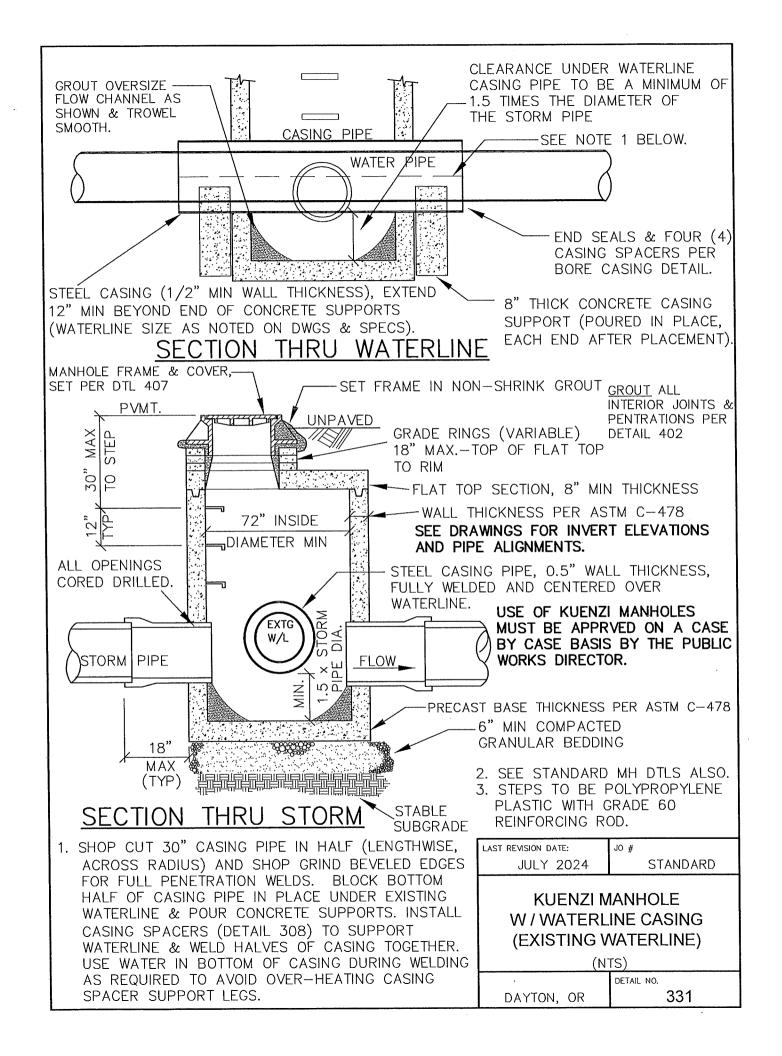


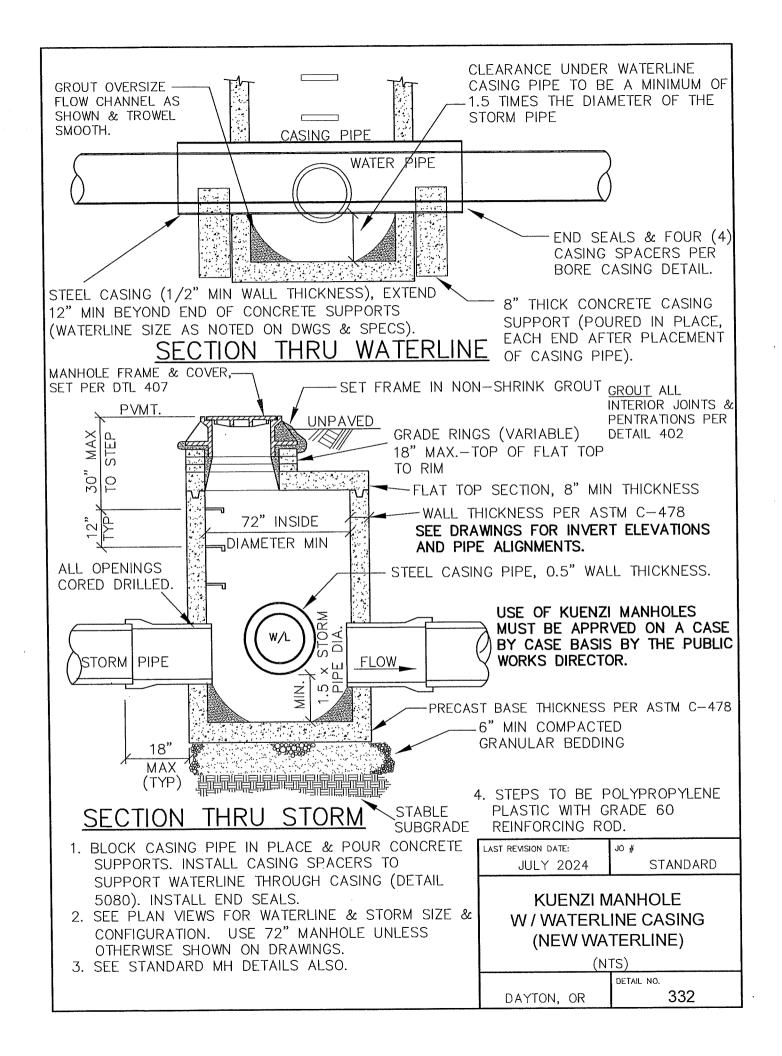


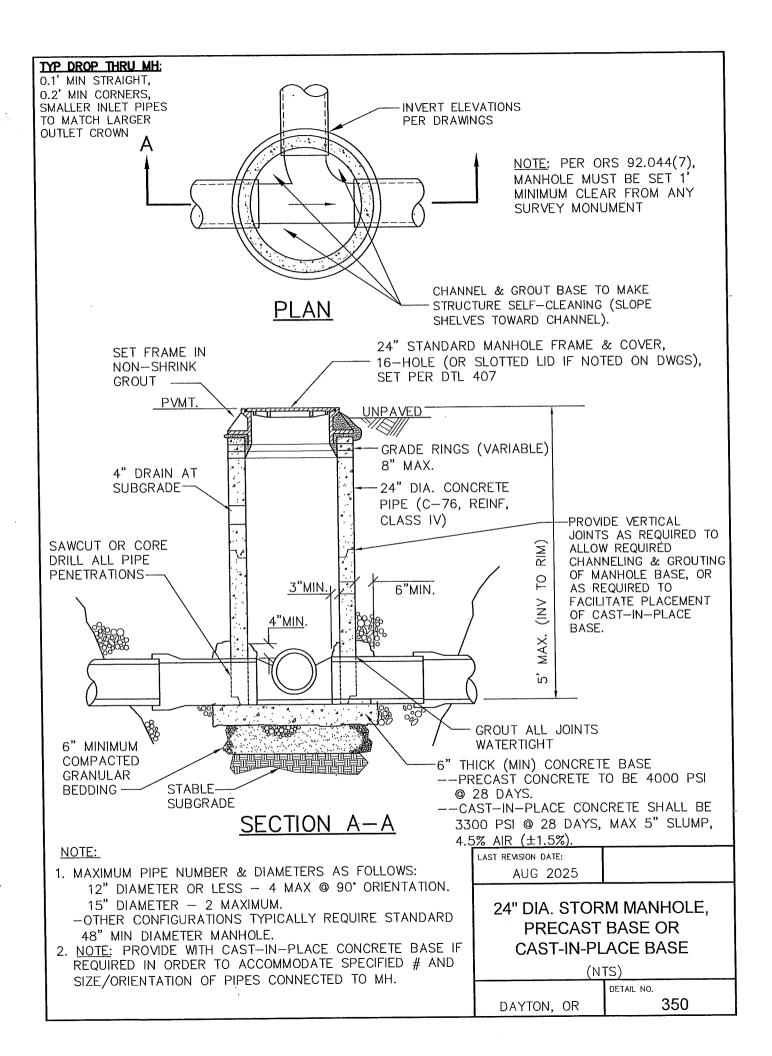


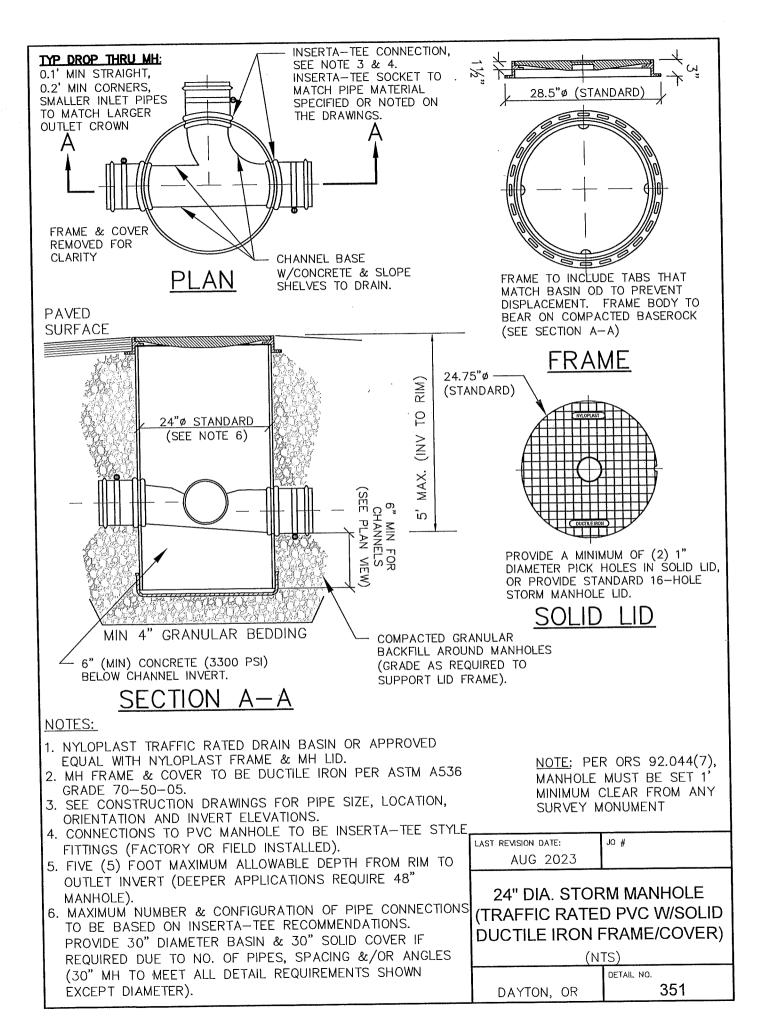


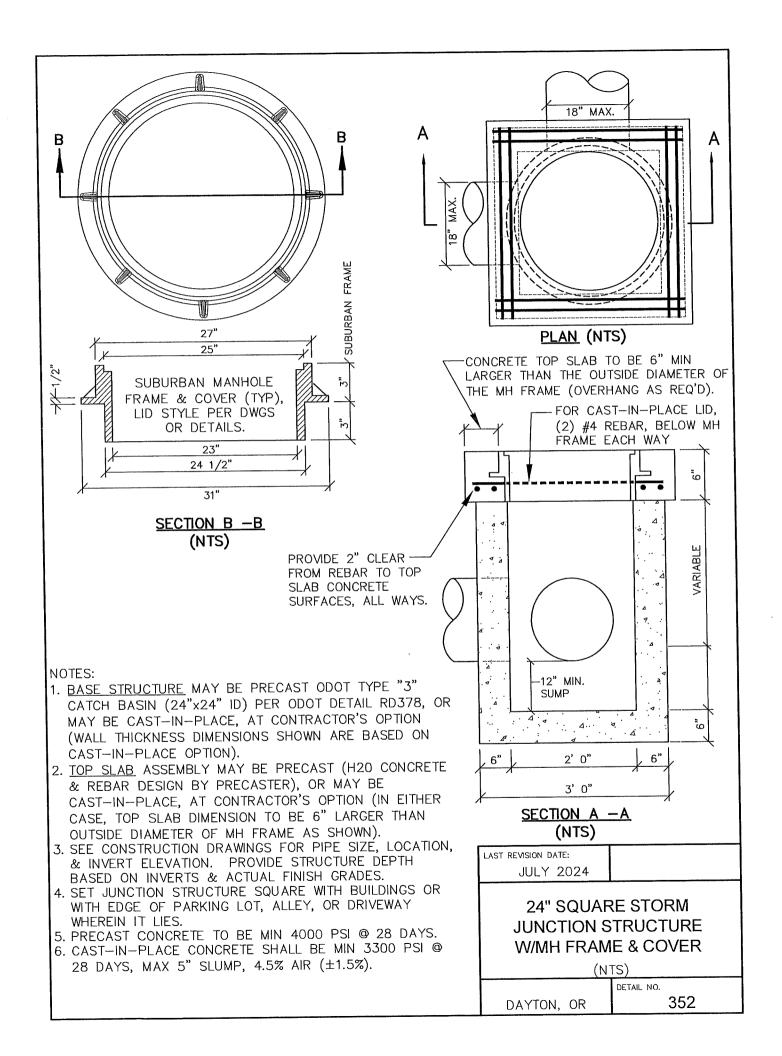


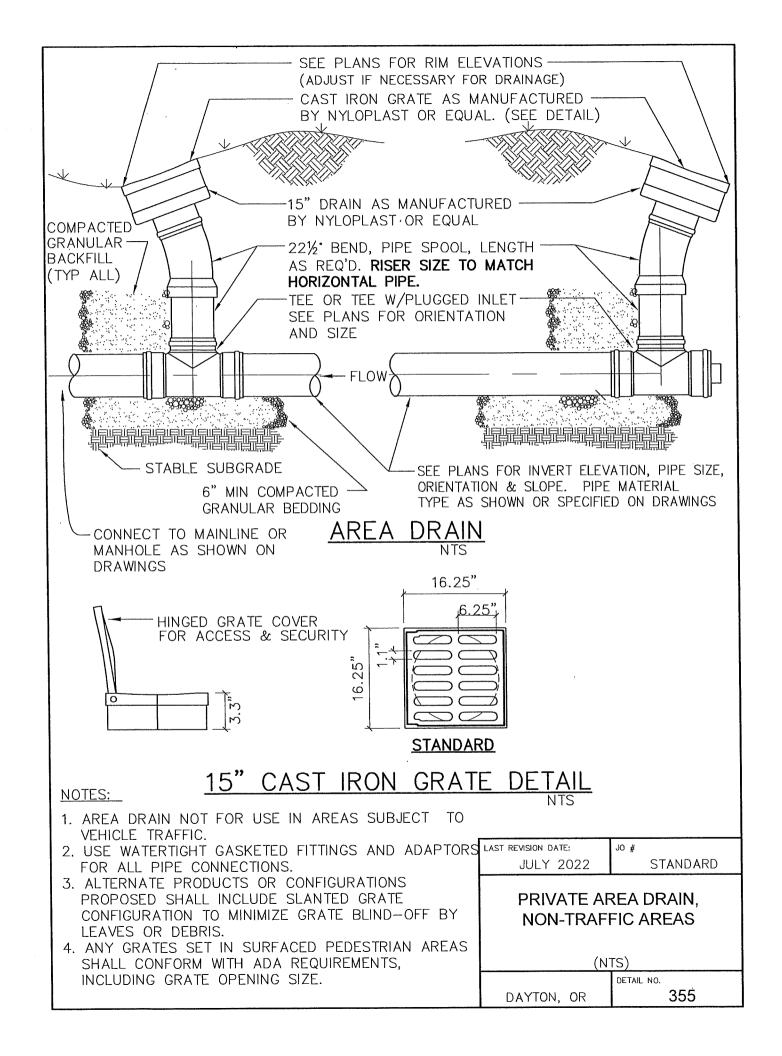


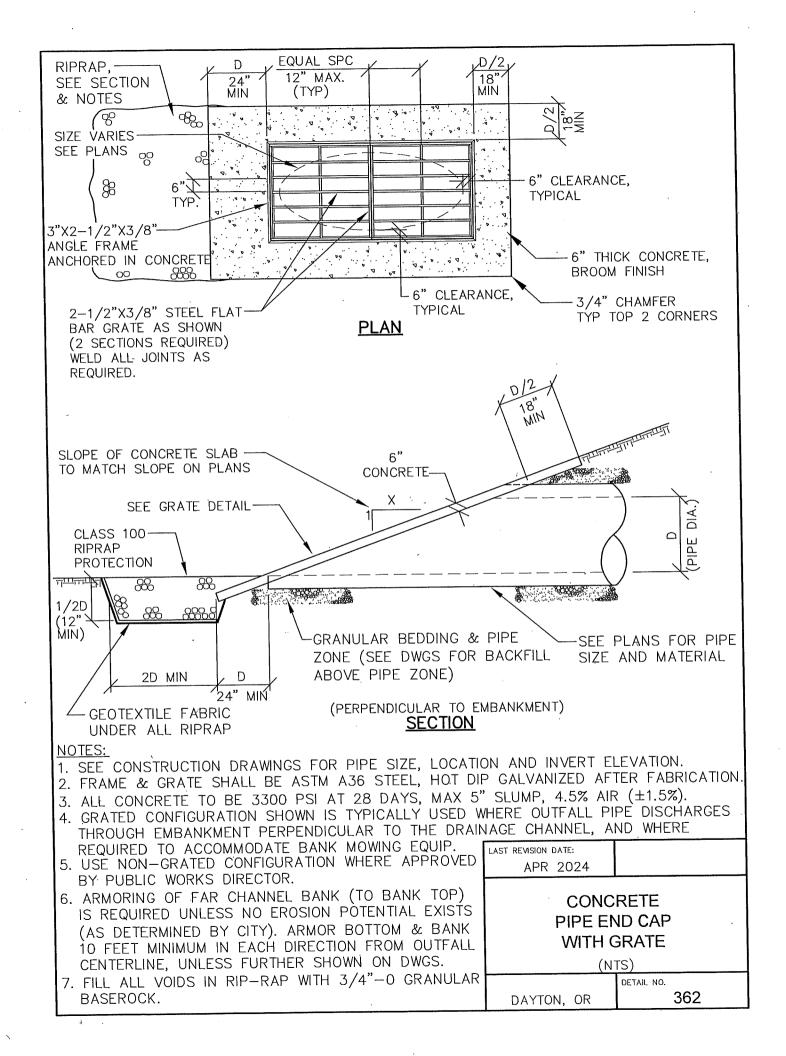


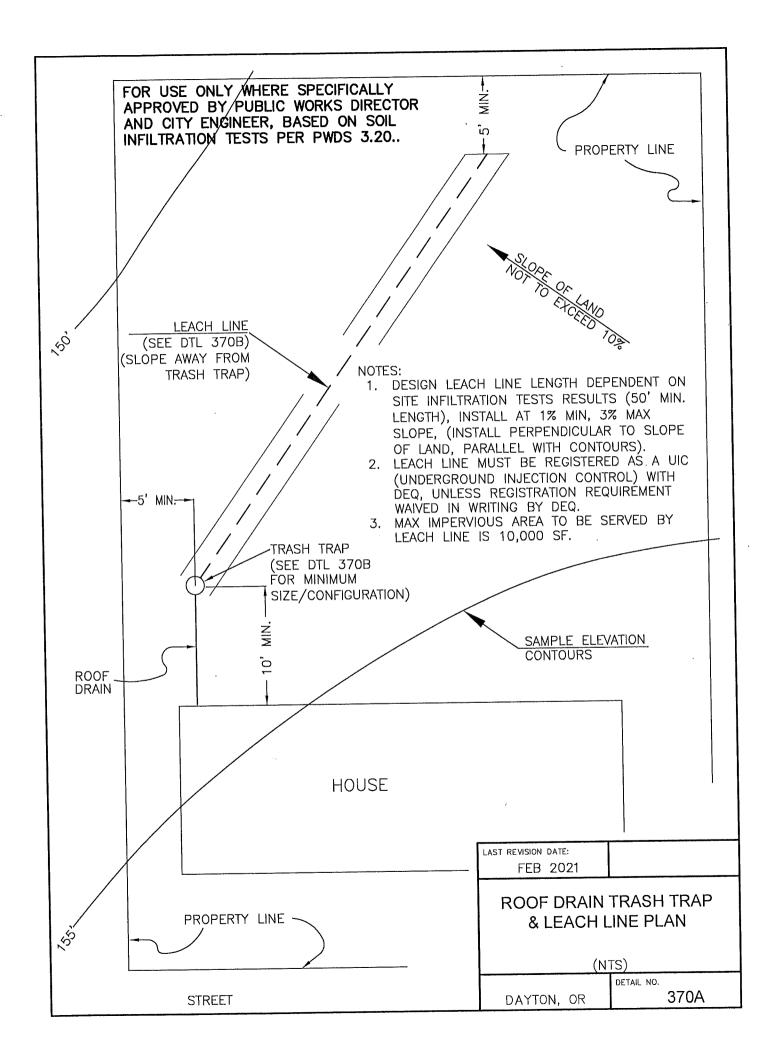




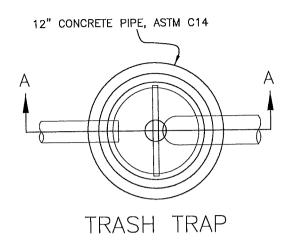


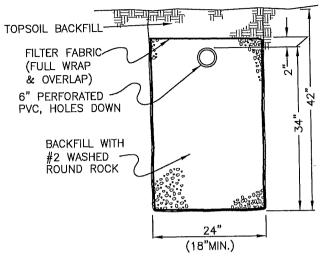




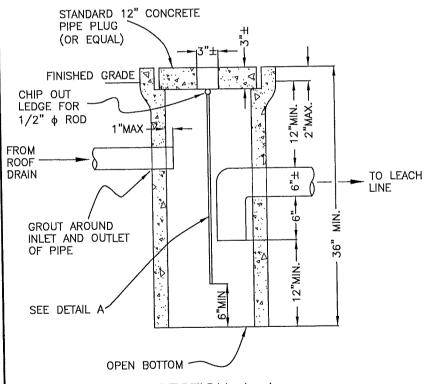


FOR USE ONLY WHERE SPECIFICALLY APPROVED BY PUBLIC WORKS DIRECTOR AND CITY ENGINEER, BASED ON SOIL INFILTRATION TESTS PER PWDS 3.20..





TYPICAL SECTION
LEACH LINE
(SEE NOTES FOR
OPTIONS)



#4 REBAR OR 1/2" \$ ROUND BAR WELD METAL PLATE 1/8" MIN. THICKNESS (OR TRAP I.D.) DETAIL A

NOTES:

SECTION A-A

- TRASH TRAP SIZE SHOWN IS MINIMUM REQUIRED BY CITY PW STANDARDS. OPSC REQUIREMENTS MAY ALSO APPLY. LARGER TRAPPED BASIN IS RECOMMENDED FOR EASE OF MAINTENANCE & CLEANING.
- 2. EZflow DRAINAGE SYSTEM by INFILTRATOR (OR EQUAL) IS ALLOWED AS AN OPTION TO WASHED ROCK TRENCH SHOWN (15" MIN BUNDLE W/PIPE).

LAST REVISION DATE: FEB 2021

TRASH TRAP & LEACH LINE DETAILS

(NTS)

DAYTON, OR

DETAIL NO. 370B

STORM SEWER MANDREL TEST REPORT

Project Location: (City)	Project Name:
Inspector: (Print)	Date: (Separate Report Required for Each Test Session)
Mandrel Diameters Verified? Yes / No	

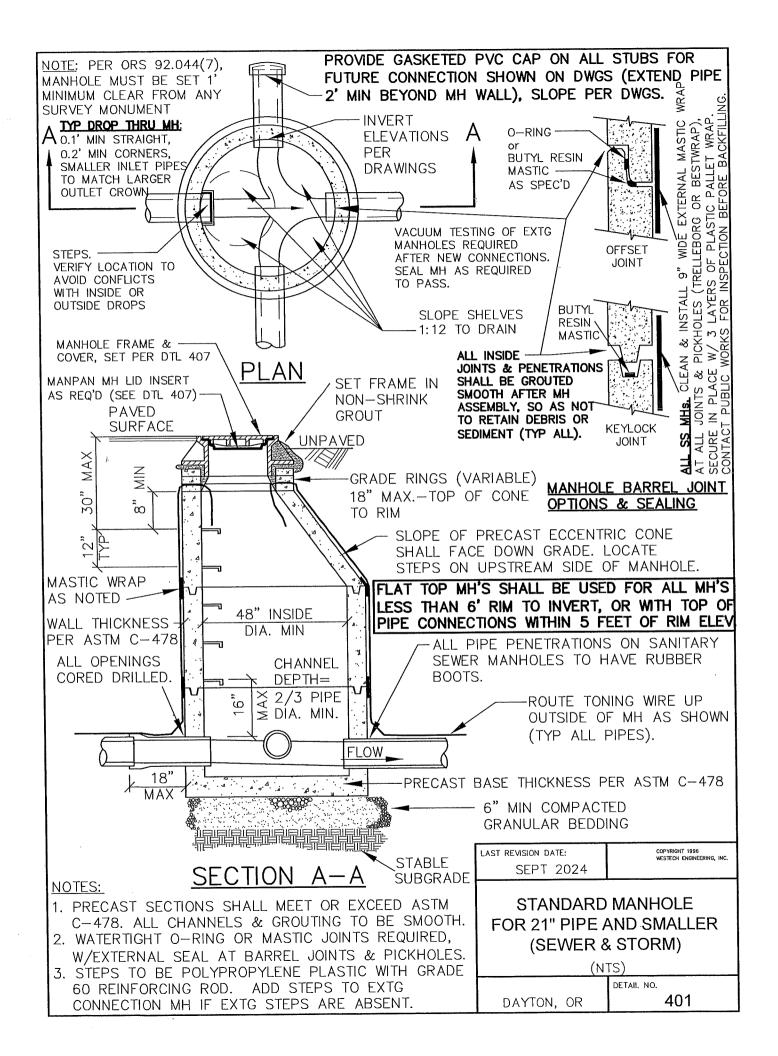
Station (& Manhole #)		Size & Material	Length (ft)	Results	Backfill Compaction Completed?	Date Sewer Flushed & Cleaned	Comments
From	То	TVIatCI Iai	(10)		Compicioa:	Steamou	
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
200				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		

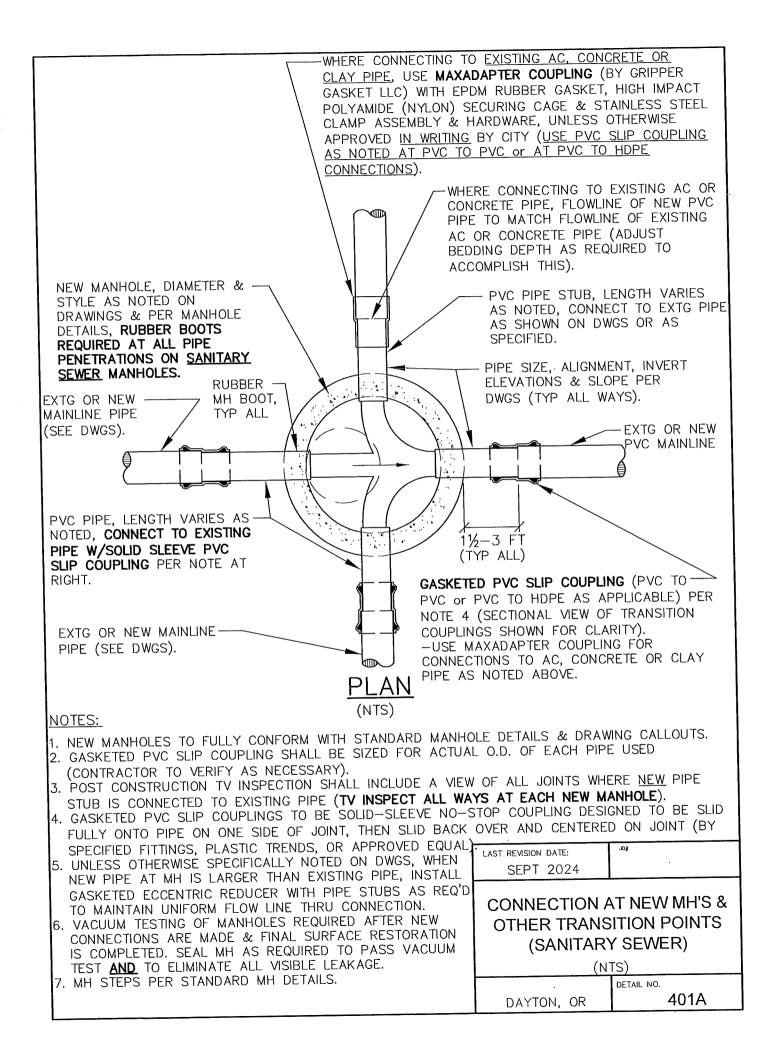
- 1. Mandrel testing shall be conducted on a manhole to manhole (or cleanout) basis and shall be done after the line has been completely flushed out with water.
- 2. Mandrel testing shall be conducted after trench backfill and compaction has been completed.
- 3. The mandrel diameter shall be 95% of the pipe initial inside diameter. The inspector shall verify the diameter of each mandrel used during each test session.

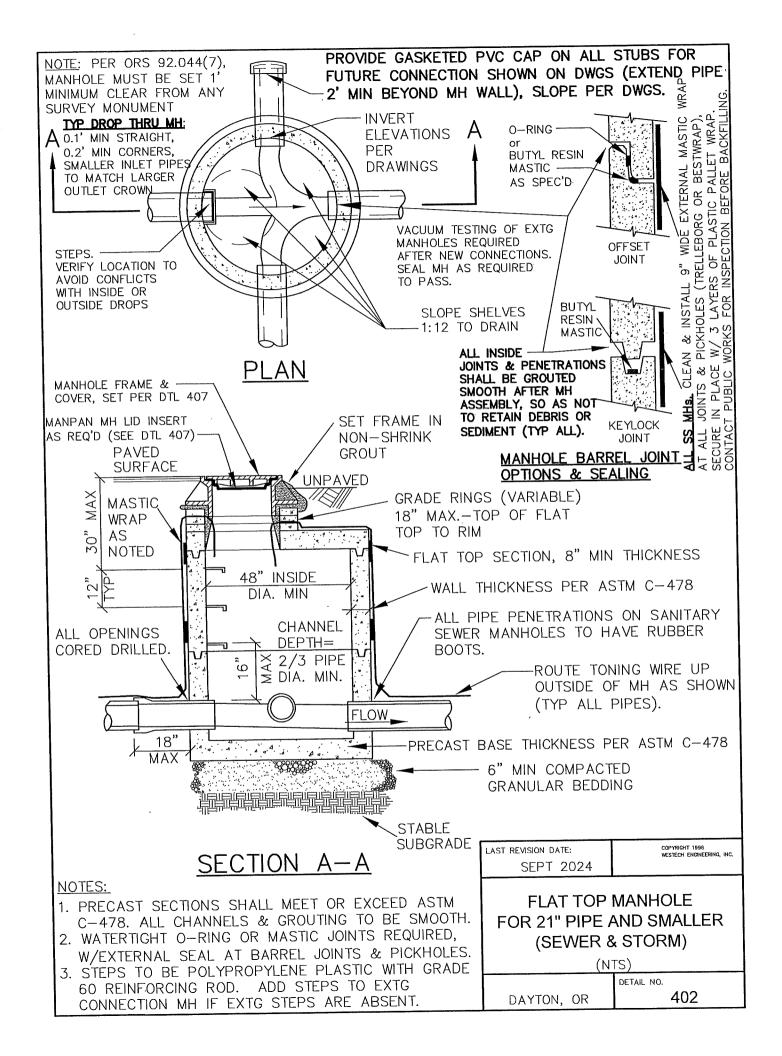
STORM PIPELINE TV INSPECTION REPORT (sample) Page of Basin No. Date: Client: City: Cleaned By: Report No. Tape No. Technician: Inspector: Weather: Pipe Material To M.H. #: From M.H. #: Pipe Dia. (in) Joint Length (ft) Section Length (ft) Joint Type: Street: Street: PIPELINE DATA: I/I (gpm) Problem Comments Cleanliness: Footage Code Alignment: Grade: %Est. Leaking Joints: Other: PROBLEM CODE LEGEND: BP = Broken Pipe CC = Circumferential Crack LC = Longitudinal Crack G = Break in GradeL = LeakPJ = Pulled Joint PT = Protruding Tap ST = Service Tap SL = Service Left SR = Service Right RT = RootsU = Unpassable PIPE MATERIAL LEGEND: AC = Asbestos Cement CIP = Cast Iron Pipe C(M) = Conc., Mortor Joint C(R) = Conc., Rubr. Gasket Jnt DI = Ductile Iron Pipe PVC = Polyvinylchloride Pipe TC = Terra Cotta VC = Vitrified Clay

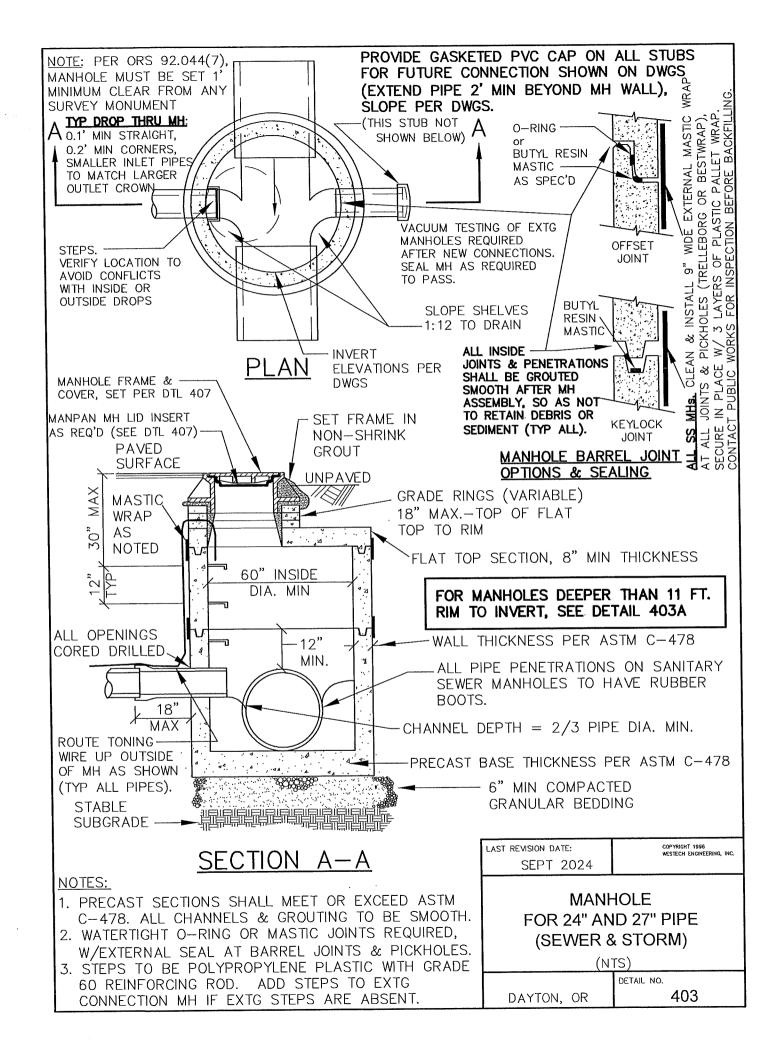
Storm TV Inspection. Upon completion of all storm drain construction, testing and repair (including channeling of storm manholes), the Contractor shall conduct a color TV acceptance inspection of all mainlines in accordance with OSSC (ODOT/APWA) 445.74 to determine compliance with grade requirements of OSSC (ODOT/APWA) 445.40.b (no deviation greater than 1/32-inch per inch of pipe diameter [1/2-inch max for pipes > 16-inch diameter], AND no reverse sloping pipe) AND to verify pipelines are adequately cleaned. The TV inspection shall be conducted by an approved technical service which is equipped to make audio-visual recordings of the TV inspections on USB storage device. Unless otherwise required by agency with jurisdiction, a standard 1-inch diameter ball shall be suspended in front of the camera during the inspection. Sufficient water to reveal low areas or reverse grades shall be discharged into the pipe immediately prior to initiation of the TV inspection. The USB storage device and written report shall be delivered to the City.

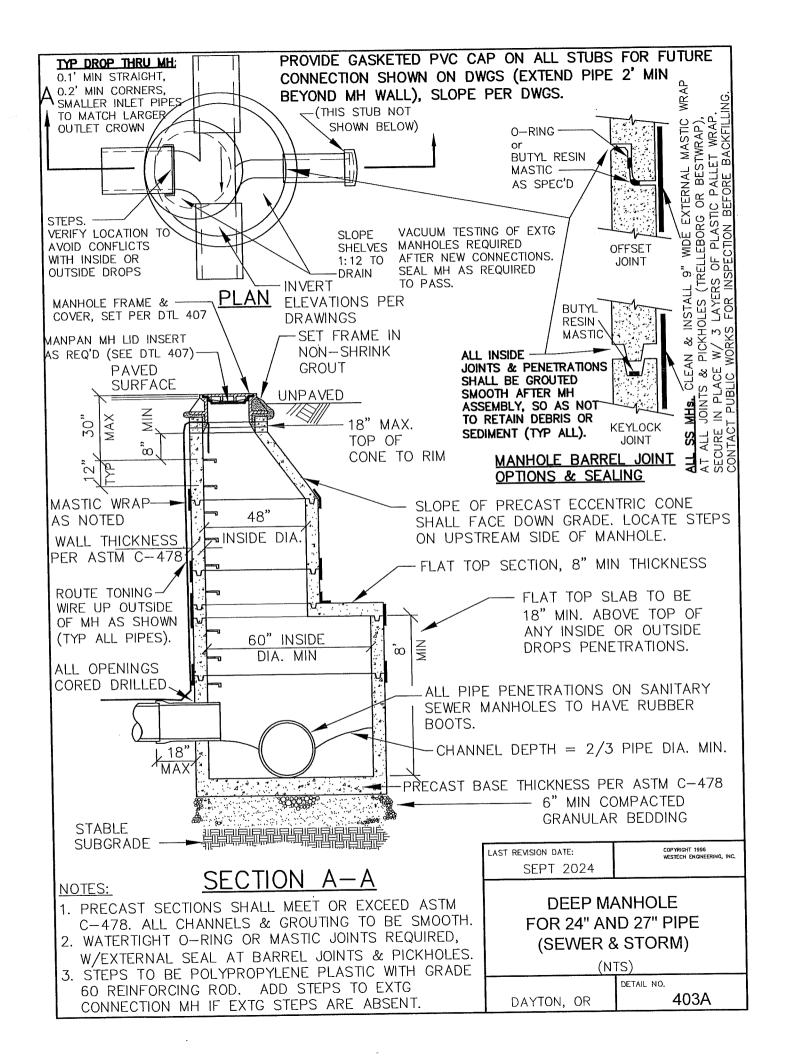
TURNAROUND: Requested (Date/time):_ Authorized (Date/time):_

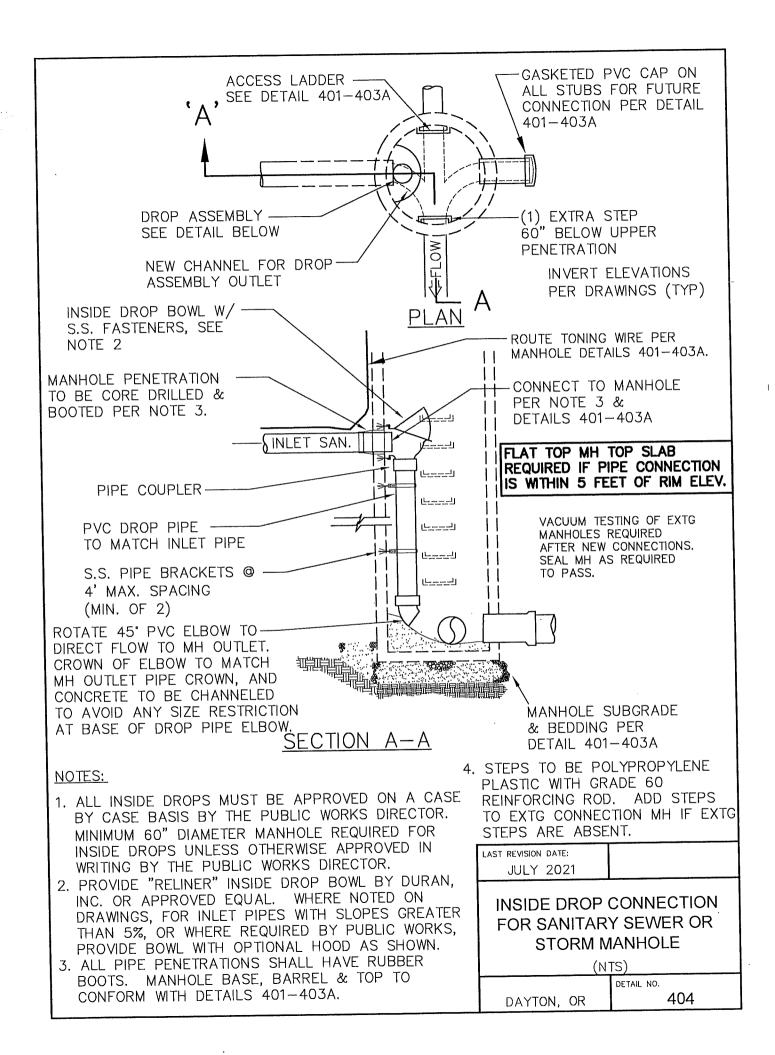


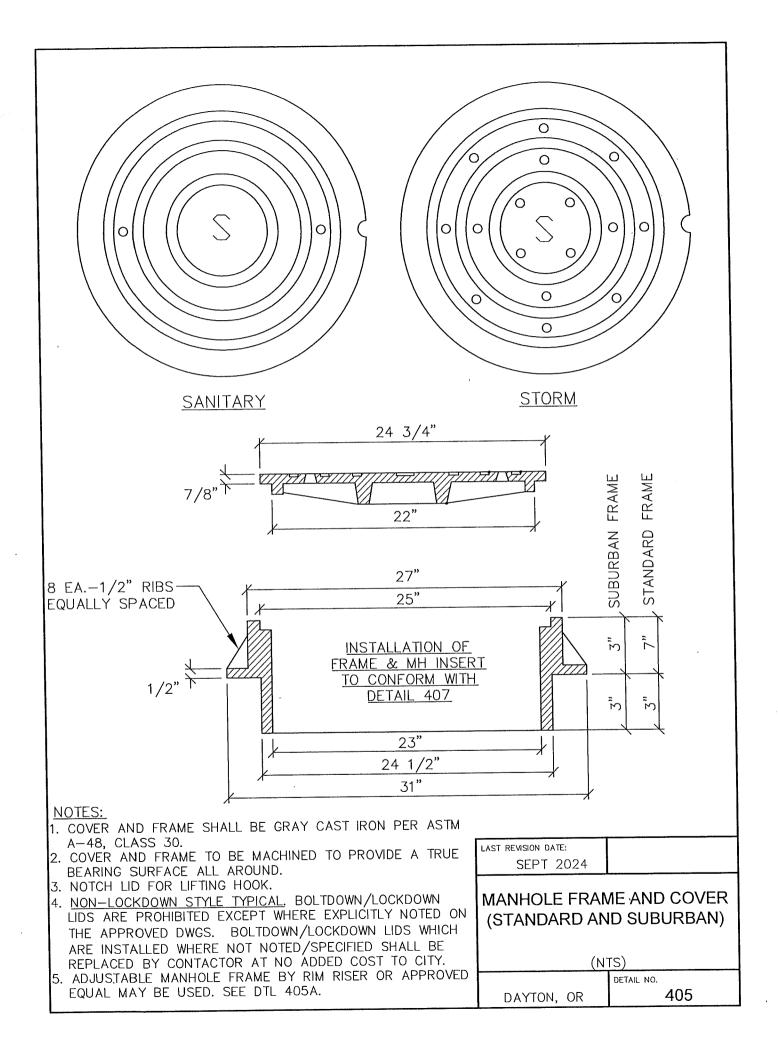


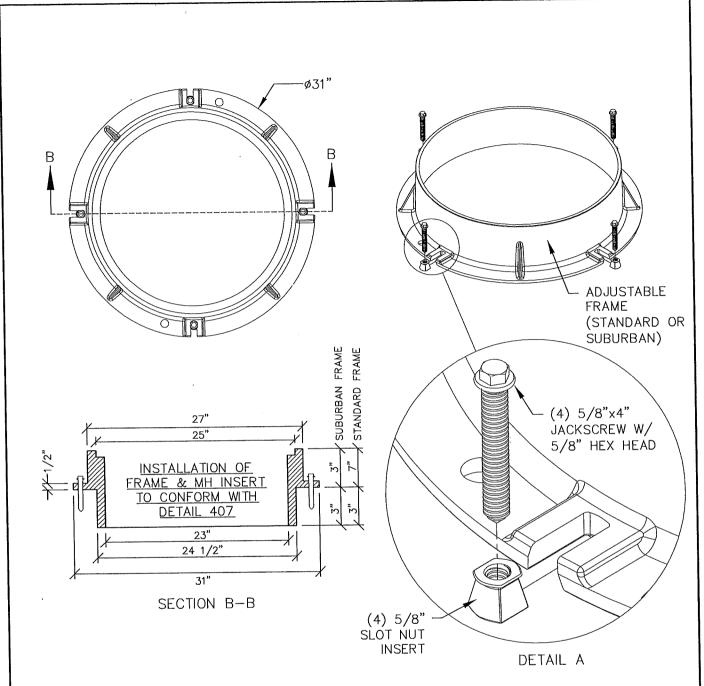












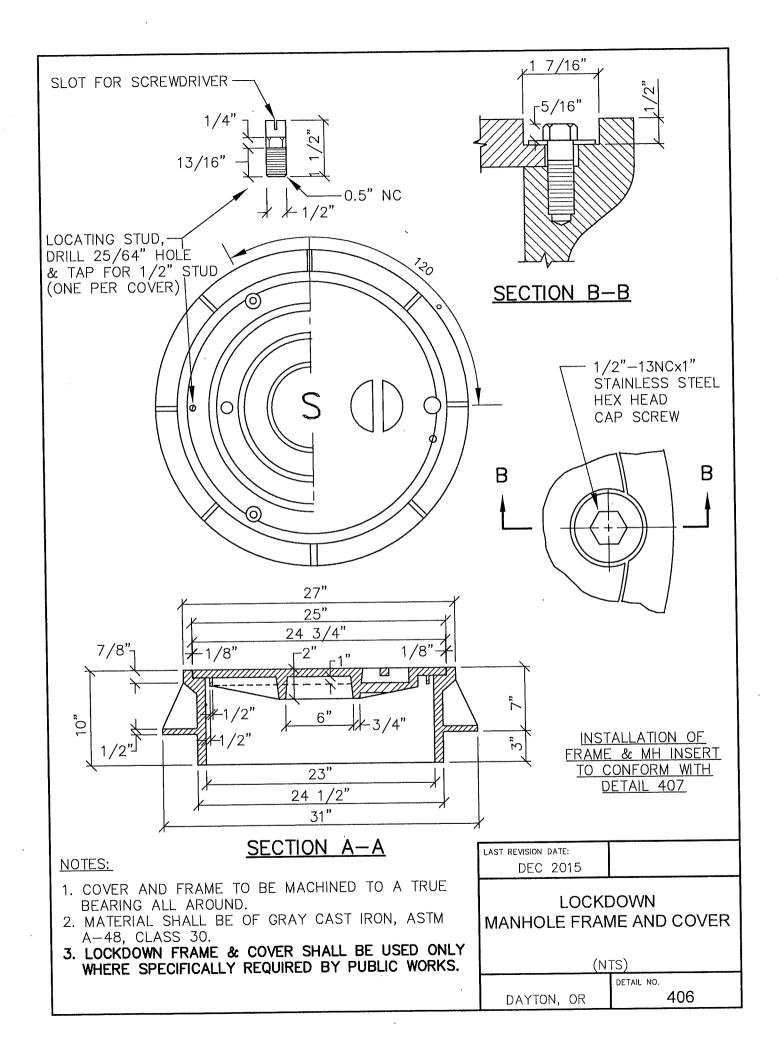
NOTES:

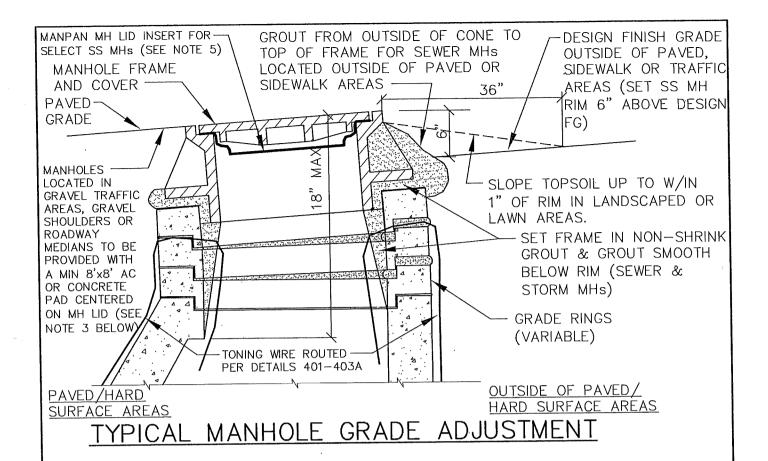
- 1. ADJUSTABLE MANHOLE FRAMES MUST BE SHOWN ON THE DESIGN DRAWINGS OR AS-BUILT DRAWINGS.
- 2. NO SHIMS REQUIRED ADJUST SCREWS TO MEET FINISH GRADE.
- 3. CASTING ASSEMBLY: AASHTO M-306 CERTIFIED, H-20 OR "TRAFFIC-RATED".
- 4. CASTINGS: GRAY IRON CONFORMS TO ASTM A48 CL35B.
- SCREWS: ZINC PLATED, MILD STEEL CONFORMS TO ASTM A1018.
- 6. NUTS: ZINC ALLOY CONFORMS TO ASTM C41A.
- 7. FILL AND PACK GAP BETWEEN FRAME AND SUPPORTING BASE WITH NON-SHRINK GROUT AND FINISH SMOOTH/FLUSH WITH INTERIOR AND EXTERIOR OF ADJOINING SURFACES PER DETAIL 4070.
- 8. MANUFACTURER TO BE RIMRISER OR APPROVED EQUAL.
- 9. USE ONLY PARTS PROVIDED BY THE MANUFACTURER.
- 10. SEE DETAIL 405 FOR MANHOLE LID (SEWER OR STORM).

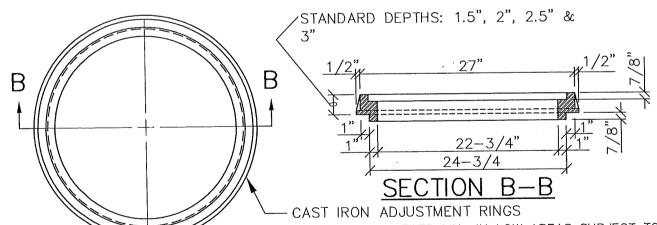
ADJUSTABLE MANHOLE
FRAME (RIM-RISER)

(NTS)
DAYTON, OR

405A







MANHOLE ADJUSTMENT RINGS FOR RESURFACING ONLY

NOTES:

1. CAST IRON ADJUSTMENT RINGS ALLOWED ONLY WITH OVERLAYS AND NOT ON NEW MANHOLES. MAXIMUM 1 ADJUSTMENT RING PER MANHOLE.

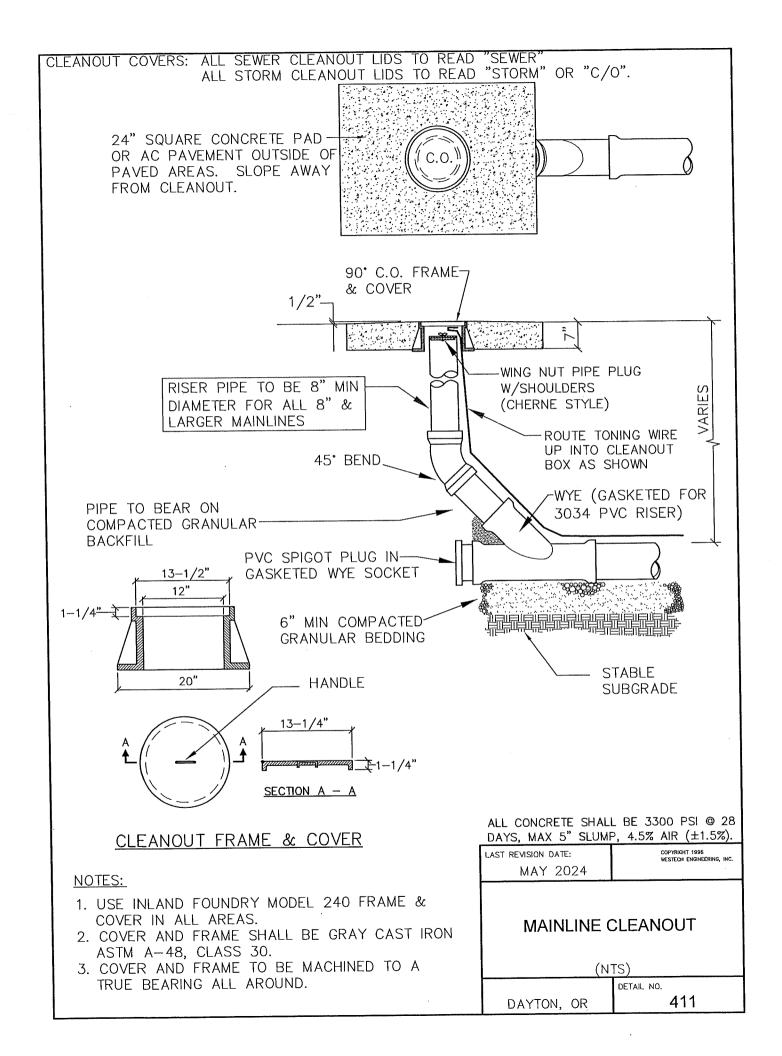
2. SANITARY SEWER MHs - 2 HOLE LIDS STORM DRAIN MHs - 16 HOLE LIDS

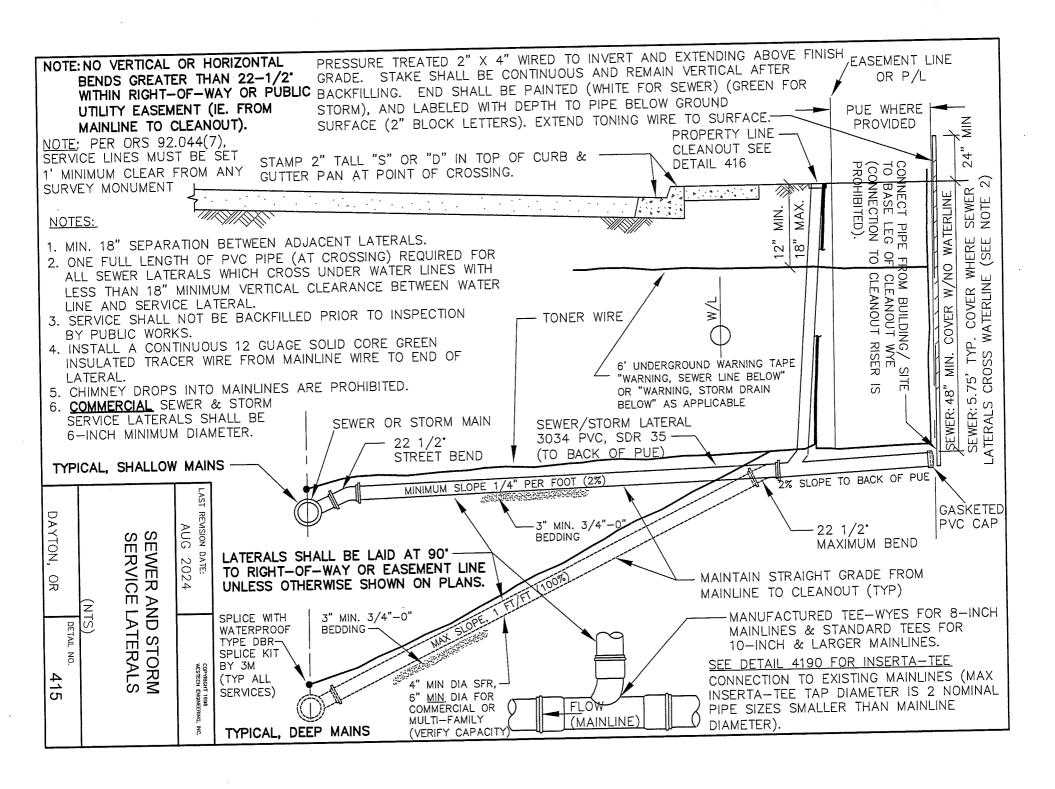
3. MH PADS IN UNPAVED TRAFFIC AREAS (OR FLOW CONTROL MH) - 8'x8' MIN SIZE OF (A) 3" MIN. AC OVER 10" COMPACTED BASEROCK (OR PUBLIC ROAD STANDARD THICKNESS IF LOCATED IN R.O.W), OR (B) 8" CONCRETE OVFR 2" BACKROCK.

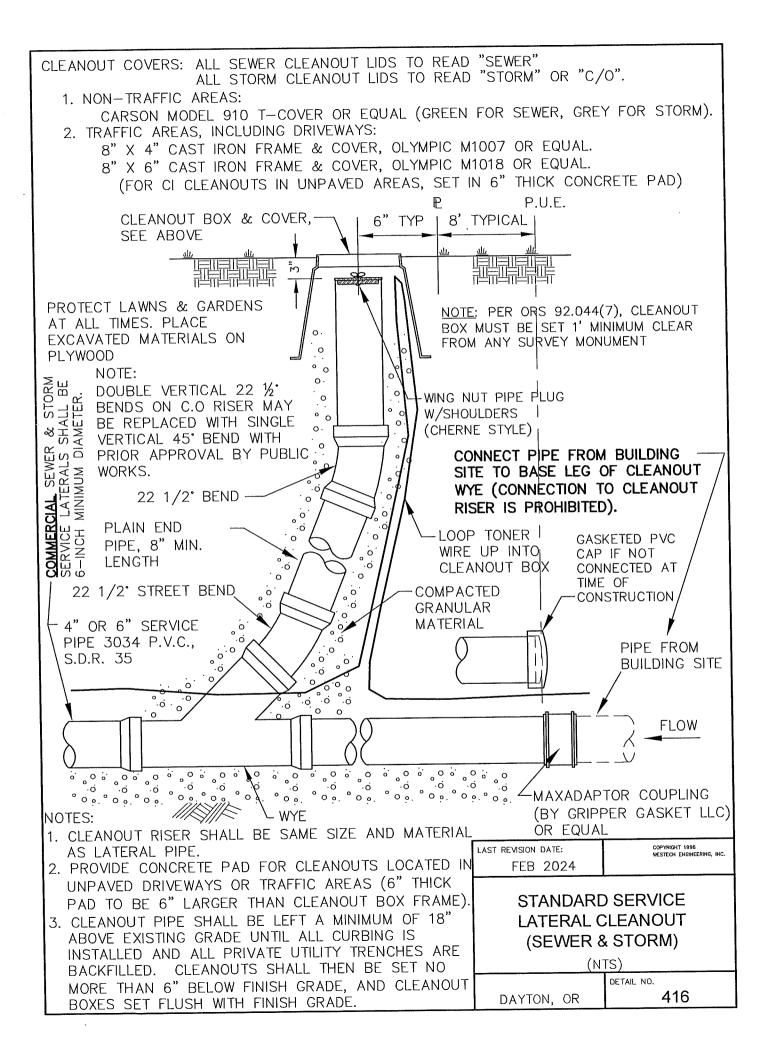
4. MH PADS IN ROAD MEDIAN PLANTER AREAS — 4" CONC (PER DTL 212, 10' MIN SQUARE W/5' SCORING PATTERN).

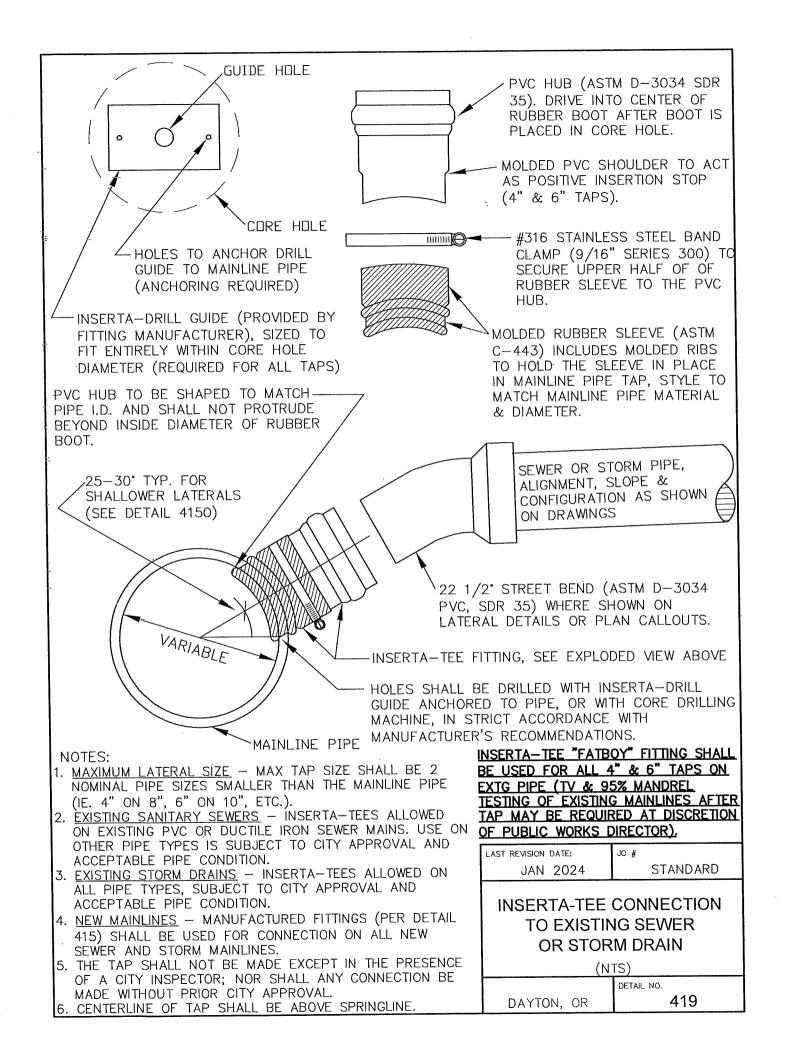
5. SEWER MHs IN LOW AREAS SUBJECT TO FLOODING OR WATER PONDING, ADJACENT TO CURBLINES OR DITCHES, ETC. SHALL BE PROVIDED WITH INFLOW PROTECTOR LID INSERTS (MAN PAN OR EQUAL). SEE CITY STANDARD CONSTRUCTION NOTES FOR LOCATION CRITERIA.

> LAST REVISION DATE: JO # AUG 2022 MANHOLE RIM ADJUSTMENT DETAILS (SEWER & STORM) (NTS) DETAIL NO. 407 DAYTON, OR









MANHOLE VACUUM TEST REPORT

Project Location: (City)						Project Name:			
Inspector: (Print)				Date (Sepa	e: trate Report Required for Eac	n Test Session)			
Testing Company: (Name & Phone #)									
Manhole No.	Manhole Diameter (inch)	Manhole Depth (ft)	Surface Restoration Complete?	Time Required (sec)]3	Time to Drop from 10" Hg to 9" Hg (sec)	Results	Comments	
			Yes / No	-			Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		
			Yes / No				Pass / Fail		

- All adjacent surface restoration shall be completed prior to conducting manhole acceptance tests, including finish paving and final adjustments to grade. Any test conducted prior to completion of surface restoration shall be considered informal, and will not count for acceptance.
- 2. The vacuum test head seal shall be inflated in accordance with the manufacturer's recommendations, but in all cases the grade rings and casting shall be included in the test. A vacuum of 10-inches of mercury shall be drawn and the vacuum pump shut off. With the valves closed, the time shall be measured for the vacuum to drop to 9-inches.
- 3. The manhole shall pass if the time for the vacuum reading to drop to 9-inches meets or exceeds the values indicated on the following table. Times for deeper depths as required by the City Engineer. Note: Visible groundwater infiltration or leakage constitutes a failed test.

REQUIRED MANHOLE VACUUM TEST TIMES									
Manhole Depth	Required Time (sec)								
(feet)	48-inch diameter	60-inch diameter	72-inch diameter						
8	20	26	33						
10	25	33	41						
12	30	39	49						
14	35	46	57						
18	40	52	65						
20	45	59	73						
22	50	65	81						

SANITARY SEWER AIR TEST REPORT

Project Location:					Project Name:					
Inspector: (Print)							Date: (Separate Report Required for Each Test Session)			
TV Inspection Required? Yes / No							el Testing Co			
X7	1 a ovv 1 - + 1	a and asses	inted alan	outs install	led and	Date C	ompleted or S	Scheduled:	which cross	sewer laterals have
cleanout riser	ll sewer lateral rs are visible a	t or above f	inish grade	? Yes/N	Io and	been in	stalled and tr	enches back	filled? Yes	/ No
Stat (& Man		Main/	Size &	Total Length	C ₁ .	K ¹	Test Time (£	Seconds) for Pre Shown (psi)	ssure Drop	Comments
From	То	Lateral	Material	(ft)			Required ²	4.0 - 3.5	3.5 - 2.5	
		Main								Pass / Fail
		Laterals								
		Totals			<u> </u>					
		Main								Pass / Fail
		Laterals								
		Totals								
		Main								Pass / Fail
		Laterals								
		Totals								
		Main					1			Pass / Fail
		Laterals			-		,			-
		Totals	-		1					

TEST PROCEDURE

- 1. Add air slowly to the portion of the pipe installation under test until the internal air pressure is raised to 4.0 psig (or higher pressure as required to address groundwater). Increase the test pressure by 0.433 psi for each foot of average ground water depth over the exterior crown of the pipe under test, with the maximum test pressure not to exceed 9.0 psi.
- 2. Add air slowly until the internal air pressure is raised to 4.0 psig (or higher pressure as required due to groundwater).
- 3. After required test pressure is reached, allow 2-minutes minimum for air temperature to stabilize, adding only the amount of air required to maintain pressure.
- 4. After the temperature stabilization period, disconnect the air supply.
- 5. Record the time required for the internal air pressure to drop from 3.5 psi (or higher as required due to groundwater backpressure) to 2.5 psi (or higher as required due to groundwater backpressure). If this time exceeds the required time (or if there is less than 1.0 psi pressure drop), the test is successful.

ACCEPTANCE: The tested sewer section shall be considered acceptable if the pressure drop during the test time is less than 1.0 psi from the starting pressure.

¹ For C and K values, see table and formulas on reverse side.

² For total $C \le 1.0$, test time (seconds) required = 2 times K

For total C > 1.0, test time (seconds) required = 2 times (K/C)

SEWER AIR TEST C AND K VALUES

Pipe Size (inch)	C-Value ¹ per foot length	K-Value ² per foot length
4	0.00155	0.176
6	0.00233	0.396
8	0.00311	0.704
10	0.00388	1.100
12	0.00466	1.584
15	0.00582	2.475
18	0.00699	3.564
21	0.00815	4.851

 $^{^{1}}$ C = 0.0003882dL

Where d = diameter (inches)

 2 K = $0.011d^{2}$ L

L = Length (ft)

Example:

Air Test a system consisting of two mainline segments as follows:

Segment 1: 395 feet of 8-inch mainline, 100 feet of 4-inch laterals, and 35 feet of 6 inch laterals. Segment 2: 200 feet of 8-inch mainline, 30 feet of 4-inch laterals, and 20 feet of 6 inch laterals.

Station (& Manhole #)		Main/ Size &		Total Size & Length	C^1	K¹	Test Time (Seconds) for Pressure Drop Shown (psi)			Comments
From	То	Lateral	Material	(ft)			Required ²	4.0 - 3.5	3.5 - 2.5	
0+00 MH A1	3+95 MH A2	Main	8" PVC	395	1.227	278.1	310/1.46 = 212			Pass / Fail
		Laterals	4" PVC 6" PVC	100 35	0.155 0.082	17.6 13.86	212*2= 414 sec			
		Totals			1.464	309.54				
3+95 MH A2	5+95 MH A3	Main	8" PVC	200	0.621	140.8	2*154=			Pass / Fail
11,22,1,12		Laterals	4" PVC 6" PVC	20 30	0.047 0.047	5.28 7.92	308 sec			
		Totals			0.714	154.0				

Note: For total $C \square 1.0$, test time (seconds) required = 2 times K For total C > 1.0, test time (seconds) required = 2 times (K/C)

The tested sewer section shall be considered acceptable when tested as described herein if the section under test does not loose air at a rate greater than 0.0015 cfm per square foot of internal sewer surface.

SANITARY SEWER MANDREL TEST REPORT

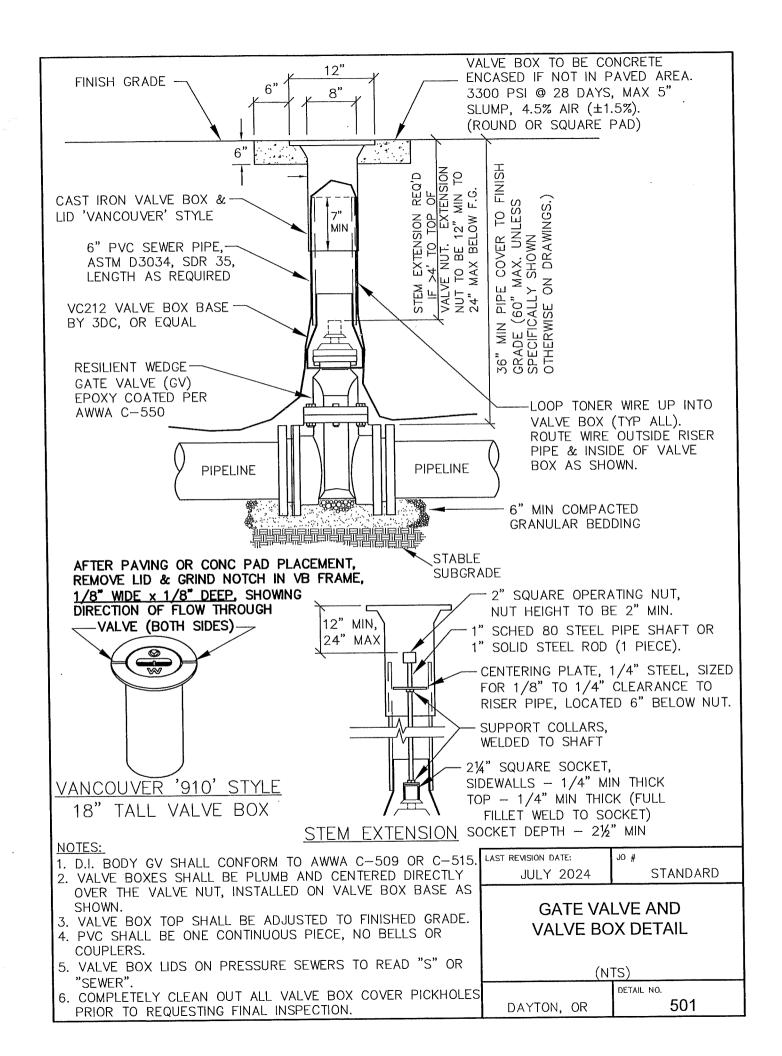
Project Location: (City)	Project Name:
Inspector: (Print)	Date: (Separate Report Required for Each Test Session)
Mandrel Diameters Verified? Yes / No	

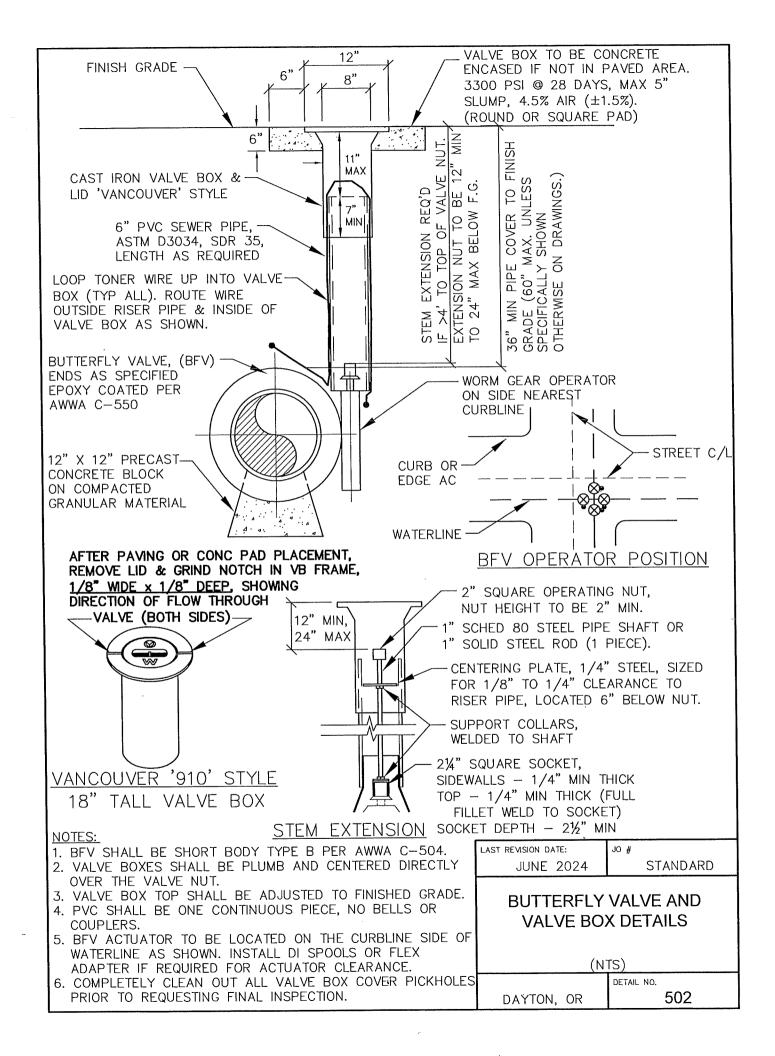
Station (& Manhole #) From To		Length (ft)	Results	Backfill Compaction Completed?	Date Sewer Flushed & Cleaned	Comments
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
 			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		
			Pass / Fail	Yes / No		

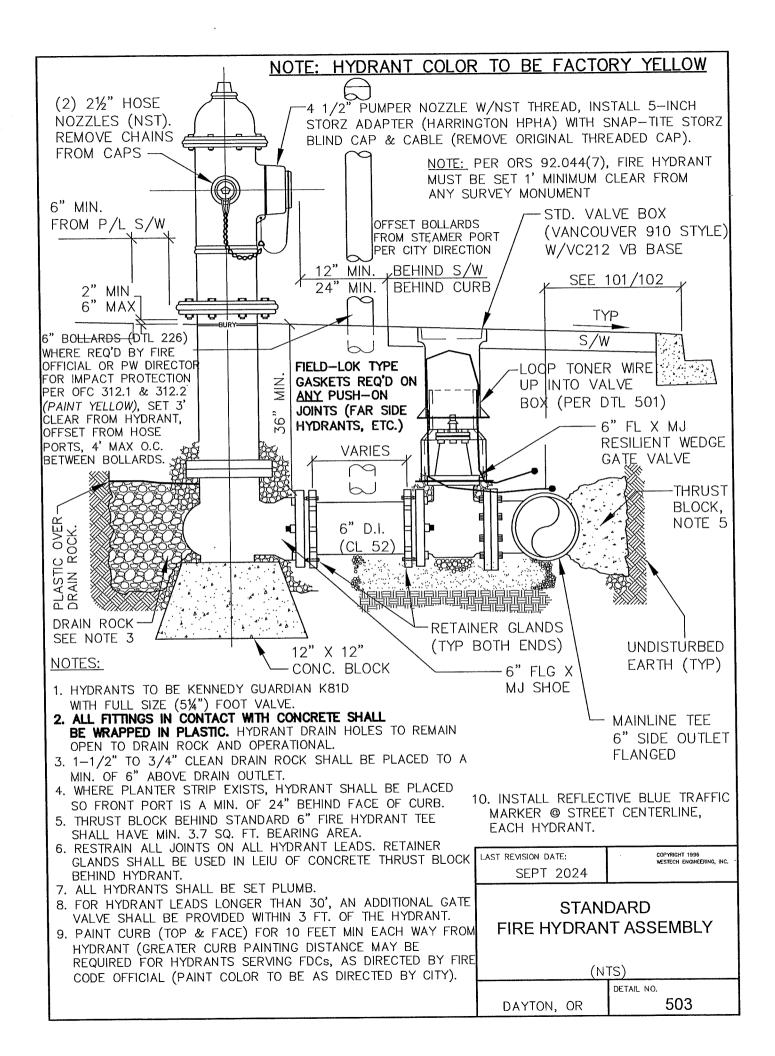
- 1. Mandrel testing shall be conducted on a manhole to manhole (or cleanout) basis and shall be done after the line has been completely flushed out with water.
- 2. Mandrel testing shall be conducted after trench backfill and compaction has been completed.
- 3. The mandrel diameter shall be 95% of the pipe initial inside diameter. The inspector shall verify the diameter of each mandrel used during each test session.

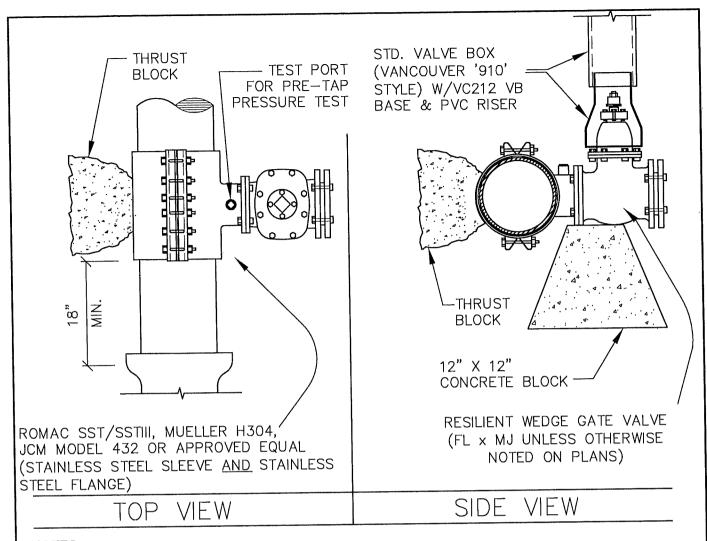
GRAVITY SEWER PIPELINE TV INSPECTION REPORT (Sample)Page Basin No. Client: Date: City: Report No. Tape No. Weather: Cleaned By: Inspector: Technician: To M.H. #: Pipe Material Joint Length (ft) Section Length (ft) Joint Type: Pipe Dia. (in) From M.H. #: Street: Street: PIPELINE DATA; I/I (gpm) Problem Comments Footage Cleanliness: Code Alignment: Grade: %Est. Leaking Joints: PROBLEM CODE LEGEND: BP = Broken PipeCC = Circumferential Crack LC = Longitudinal Crack G = Break in Grade L = LeakPJ = Pulled Joint PT = Protruding Tap ST = Service Tap SL = Service Left SR = Service Right RT = RootsU = Unpassable PIPE MATERIAL LEGEND: AC = Asbestos Cement CIP = Cast Iron Pipe C(M) = Conc., Mortor Joint C(R) = Conc., Rubr. Gasket Jnt DI = Ductile Iron Pipe PVC = Polyvinylchloride Pipe TC = Terra Cotta VC = Vitrified Clay TURNAROUND: Requested (Date/time): Authorized (Date/time):_

Gravity Sewer TV Inspection. Upon completion of all sewer construction, testing and repair (including channeling of sanitary sewer manholes), the Contractor shall conduct a color TV acceptance inspection of all mainlines in accordance with OSSC (ODOT/APWA) 445.74 to determine compliance with grade requirements of OSSC (ODOT/APWA) 445.40.b (no deviation greater than 1/32-inch per inch of pipe diameter [1/2-inch max for pipes >16-inch diameter], AND no reverse sloping pipe), AND to verify pipelines are adequately cleaned. The TV inspection shall be conducted by an approved technical service, using a track or wheel propelled self-leveling auto-focus pan-head camera which is equipped to make audio-visual recordings of the TV inspections on a USB storage device. Unless otherwise required by the agency with jurisdiction, a standard 1-inch diameter ball shall be suspended in front of the camera during the inspection (with the ball in contact with the pipe invert) to determine the depth of any standing water. Sufficient water to reveal low areas or reverse grades shall be discharged into the pipe immediately prior to initiation of the TV inspection. The USB storage device and written report (or download link and pdf report) shall be delivered to the City Engineer.









NOTES:

- 1. WATER MAIN SHALL BE CLEANED & SPRAYED WITH CHLORINE SOLUTION IN TAP AREA BEFORE ATTACHING SLEEVE.
- 2. TAPPING SLEEVE SHALL BE ALL STAINLESS STEEL WITH FULL PERIMETER GASKET.

3. TAPPING VALVE SHALL BE EPOXY COATED PER AWWA C-550.

- 4. <u>PRE-TAP PRESSURE TEST</u>, SLEEVE AND VALVE SHALL BE PRESSURE TESTED BEFORE MAKING TAP. PRESSURE TEST AND TAP SHALL BE MADE IN THE PRESENCE OF AN AUTHORIZED WATER SYSTEM REPRESENTATIVE.
- 5. APPROVED TAPPING MACHINE SHALL BE USED TO MAKE TAP.
- 6. 3/4" GRANULAR BACKFILL SHALL BE PLACED AND COMPACTED TO 92% OF MAXIMUM DENSITY AS DETERMINED BY AASHTO T-180.
- 7. THRUST BLOCKING PER DETAIL 510.
- 8. TAP SHALL BE MADE NO CLOSER THAN 18" FROM THE NEAREST JOINT.
- 9. SLEEVE AND VALVE SHALL BE WRAPPED WITH 8 MIL PLASTIC PRIOR TO CONCRETE PLACEMENT.
- 10. CONCRETE BLOCK(S) SHALL COMPLETELY SUPPORT TAPPING TEE AND VALVE.
- 11. CONTRACTOR SHALL COORDINATE ALL TAPS WITH CITY AND PERFORM ALL TAPS WITH PUBLIC WORKS STAFF PRESENT.
- 12. ALL TAPPING EQUIPMENT (AND ANY TOOL COMING IN CONTACT WITH THE PIPE THOUGH THE TAPPING SLEEVE) SHALL BE CHLORINE DISINFECTED WITH A 300 MG/L CHLORINE SOLUTION.

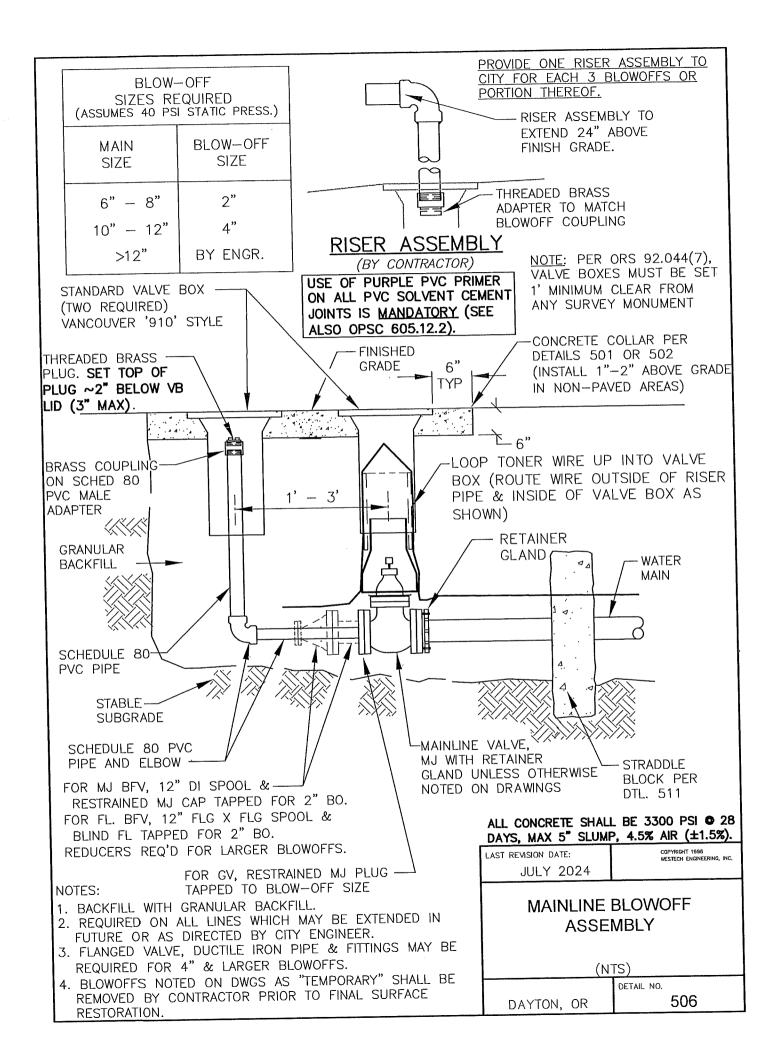
LAST REVISION DATE: COPYRIGHT 1996 WESTECH ENGINEERING, INC.

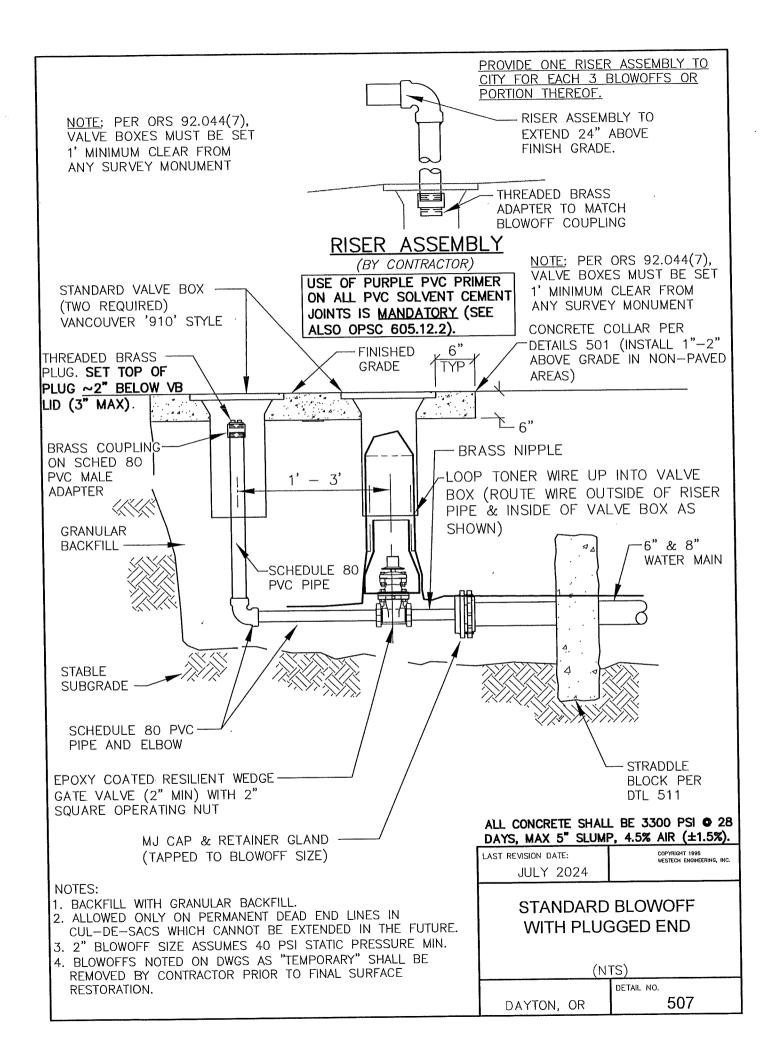
SEPT 2018

TAPPING TEE AND VALVE

(NTS)

DAYTON, OR 505





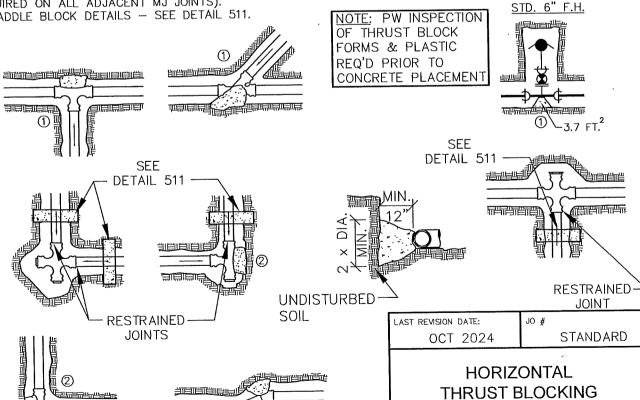
FITTING SIZE (Inches)	TEE, WYE, & ① HYDRANTS	90° BEND ② (STRADDLE BLOCKS REQ'D FOR PLUGGED CROSS OR PLUGGED TEE)	45° BEND ③	22 1/2* BEND ③	11 1/4° BEND ③	
2	*	*	*	*	*	
4	1.7	2.4	1.3	*	*	
6	3.7	5.3	2.9	1.5	*	
8	6.7	9.5	5.1	2.7	1.3	
10	10.5	14.8	8	4.1	2	
12	15.1	21.3	11.6	5.9	2.9	
16	26.8	37.9	20.5	10.4	5.2	
18	33.9	47.9	25.9	12.8	6.7	
LARGER	* *	* *	* *	* *	* *	
	BEARING AREA OF THRUST BLOCKS (sq. ft.)					

- BLOCK TO UNDISTURBED TRENCH WALLS
- * THRUST BLOCKS FOR PIPES LARGER THAN 18" WILL BE INDIVIDUALLY DESIGNED BY THE ENGINEER.
- 1. ALL VALUES ARE BASED ON THE FOLLOWING ASSUMPTIONS:

AVG. PRESSURE = 100 PS1 \times 2 (safety factor); 1500 PSF SOIL BEARING CAPACITY; NORMAL DISTRIBUTION SYSTEM DESIGN VELOCITY NOT TO EXCEED 5 FPS.

- ALL FITTINGS SHALL BE WRAPPED IN PLASTIC PRIOR TO PLACEMENT OF CONCRETE.
- 3. BEARING SURFACE OF THRUST BLOCKING SHALL BE AGAINST UNDISTURBED SOIL.
- 4. TRUCK-MIXED CONCRETE MIX SHALL HAVE A MIN. 28 DAY STRENGTH OF 3300 PSI (5" MAX SLUMP). USE OF HAND-MIXED SACK-CRETE TYPE CONCRETE REQUIRES WRITTEN LOCAL JURISDICTION APPROVAL PRIOR TO USE, AND SHALL BE 4000 PSI MIX, MIXED WITH MIN AMOUNT OF WATER NECESSARY FOR WORKABILITY (5" MAX SLUMP). USE OF DRY SACK-CRETE MIX (BAGS OR LOOSE MIX) IS PROHIBITED FOR PERMANENT THRUST
- 5. ALL PIPE ZONES SHALL BE BACKFILLED WITH GRANULAR BACKFILL AND COMPACTED.
- 6. IF THRUST BLOCKS ARE APPROVED IN WRITING FOR INSTALLATION IN FRONT OF PLUGGED CROSS OR PLUGGED TEE, EACH SHALL HAVE A #4 REBAR LIFTING LOOP INSTALLED IN TOP TO ALLOW FOR FUTURE TB REMOVAL.
- 7. <u>VERTICAL THRUST RESTRAINT</u> TYPICALLY USE STRADDLE BLOCK PER DETAIL 511 (RETAINER GLANDS





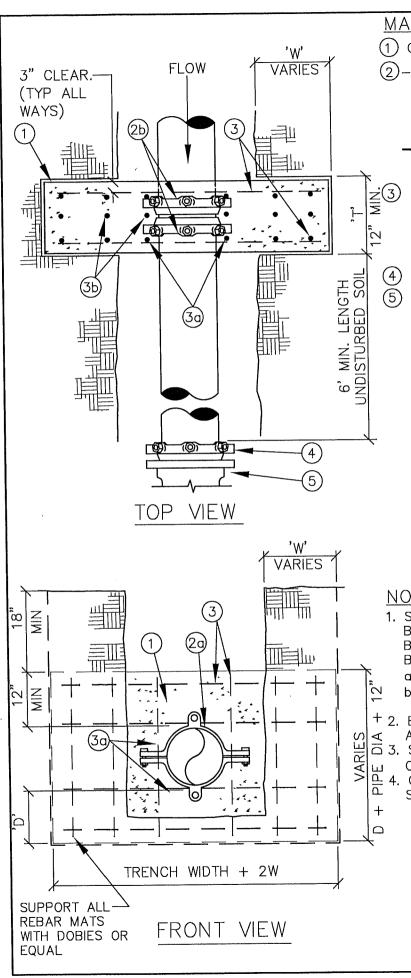
(NTS)

DAYTON, OR

DETAIL NO.

510

3



MATERIALS

- (1) CONCRETE STRADDLE BLOCK.
- 2)-EITHER (2d) ONE SERRATED-LOCK STYLE SPLIT-RING RESTRAINT HARNESS (ROMAC 600 OR EQUAL), OR (2b) TWO RETAINER GLAND WEDGE-STYLE RESTRAINTS, SET OPPOSED (EBBA MEGA-LUG OR EQUAL).

-WEDGE STYLE RESTRAINTS SHALL BE WRAPPED WITH PLASTIC PRIOR TO CONCRETE PLACEMENT.

≤12" PIPE, #4 REBAR @12" O.C. E.W., (3a) INSTALL REBAR EACH SIDE OF RESTRAINT FITTING INSIDE CONCRETE AS SHOWN. (3b) INSTALL 3 MATS OF REBAR FOR PIPE LARGER THAN 12" DIAMETER. RETAINER GLAND, ON ADJACENT FITTING.

) MJ FITTING, BEND, VALVE OR BLOWOFF.

PIPE SIZE	'W'	'D'	'T'	
6"	12" 8"		12"	
8"	16"	10"	12"	
10"	20"	12"	12"	
12"	24"	18"	18"	
14"&16"	28"	24"	18"	
18"	32"	30"	18"	
>12"	SIZE TO BE VERIFIED BY DESIGN ENG (NOTE 1).			

- 1. STRADDLE BLOCKS FOR >12" PIPE SHALL BE VERIFIED INDIVUALLY FOR APPLICATION BY THE DESIGN ENGINEER AND SHALL BE BASED ON THE FOLLOWING:
 - a.) 200 PSI WATER TEST PRESSURE.
 - b.) SOIL BEARING CAPACITY, REBAR SIZE & SPACING VERIFIED BY THE ENGINEER.
- + 2. BEARING AREA OF BLOCK SHALL BE

 ✓ AGAINST UNDISTURBED SOIL.
- 3. STRADDLE BLOCK SHALL HAVE A MINIMUM OF 18" COVER.
- □ OF 18" COVER.

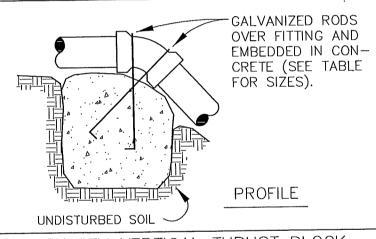
 □ 4. CONCRETE SHALL HAVE A MIN. 28 DAY
 □ STRENGTH OF 3300 PSI.

LAST REVISION DATE: DEC 2021	COPYRIGHT 1996 WESTECH ENGINEERING, INC.				
STRADDLE BLOCK FOR WATERLINE PIPE & PRESSURE SEWER PIPE					
(NTS)					
DAYTON, OR	DETAIL NO. 511				

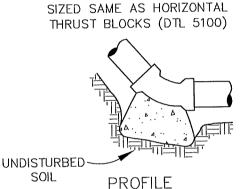
NOTICE: ANY GRAVITY VERTICAL THRUST BLOCK PROPOSED IN LIEU OF STRADDLE BLOCKS SHALL BE BASED ON A PROJECT SPECIFIC DESIGN BY THE ENGINEER (SEE PLANS) AND APPROVED BY THE LOCAL JURISDICTION. AS ADDITIONAL CORROSION PROTECTION, 6 OZ MARS ZINC CAP NUTS (SACRIFICIAL ANODES) SHALL BE INSTALLED ON THE END OF ALL BOLTS ON ALL JOINTS AT (OR ADJACENT TO) THE GRAVITY VERTICAL THRUST BLOCK LOCATION.

NOTES:

- 1. KEEP CONCRETE CLEAR OF JOINT AND JOINT ACCESSORIES. FITTINGS SHALL BE WRAPPED IN PLASTIC PRIOR TO PLACEMENT OF CONCRETE.
- 2. CONCRETE THRUST BLOCKING SHALL BE POURED AGAINST UNDISTURBED EARTH.
- 3. CONCRETE MIX SHALL BE 3300 PSI @ 28 DAYS, MAX 5" SLUMP, 4.5% AIR (±1.5%).
- 4. THRUST BLOCK VOLUMES FOR VERTICAL BENDS HAVING UPWARD RESULTANT THRUSTS ARE BASED ON TEST PRESSURE OF 150 P.S.I.G. AND THE WEIGHT OF CONCRETE = 4050 LBS./CU.YD (150 PCF).
- 5. ALL REBAR SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM-123. REBAR SHALL BE BENT BEFORE GALVANIZATION, AND LAST 4" OF BAR SHALL BE BENT 90 DEGREES WITH A 1/2" RADIUS BEND. REBAR SHALL BE TIGHTLY FIT TO RESTRAINED FITTING.
- 6. FOR HORIZONTAL THRUST BLOCK DETAILS SEE DETAIL 510.
- 7. STRADDLE BLOCK NOTE: FOR VERTICAL BENDS, STRADDLE BLOCKS ARE TYPICALLY REQUIRED IN LIEU OF GRAVITY VERTICAL THRUST BLOCKS (SEE NOTE BELOW RIGHT). SEE DETAIL 511 FOR GENERAL CONFIGURATION OF STRADDLE BLOCKS.



GRAVITY VERTICAL THRUST BLOCK



NORMAL VERTICAL THRUST BLOCK

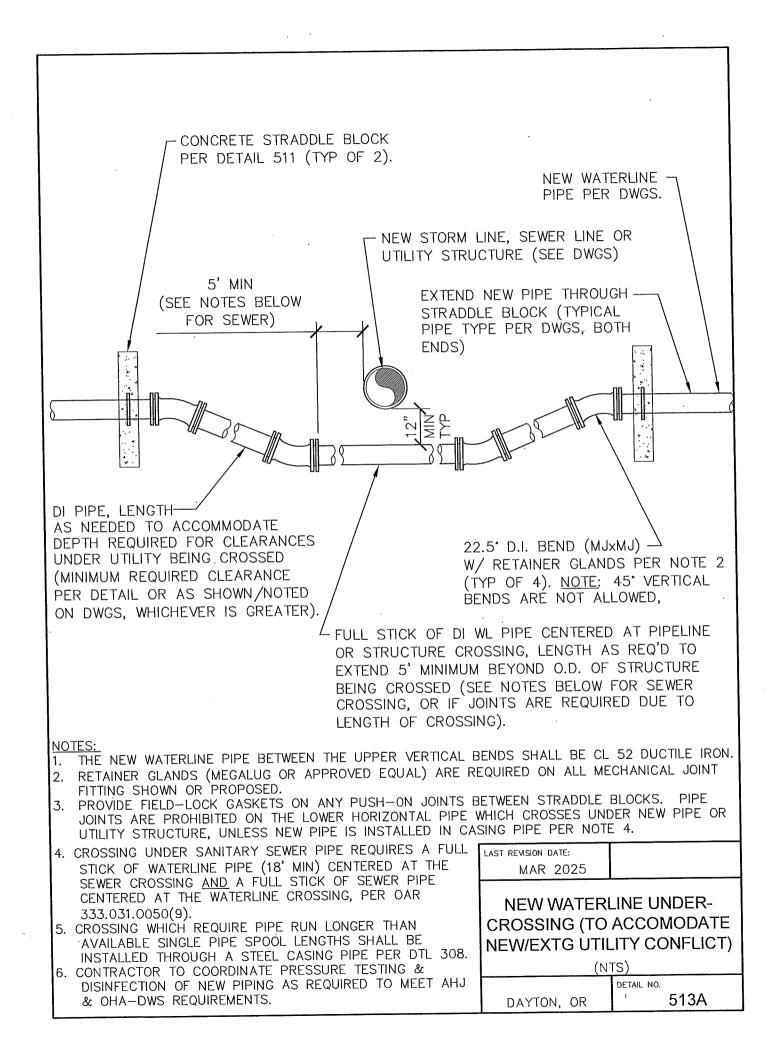
VOLUME OF THRUST BLOCK IN CUBIC YARDS (VERTICAL BENDS)					
FITTING	FITTING BEND ANGLE				
SIZE	45° 22 1/2° 11 1/4°				
4	1.1	0.4	0.2		
6	2.7	1.0	0.4		
8	8 4.0		0.6		
10	***	2.3	0.9		
12 ***		3.2	1.3		
14	***	4.3	1.8		
16	***	***	2.3		

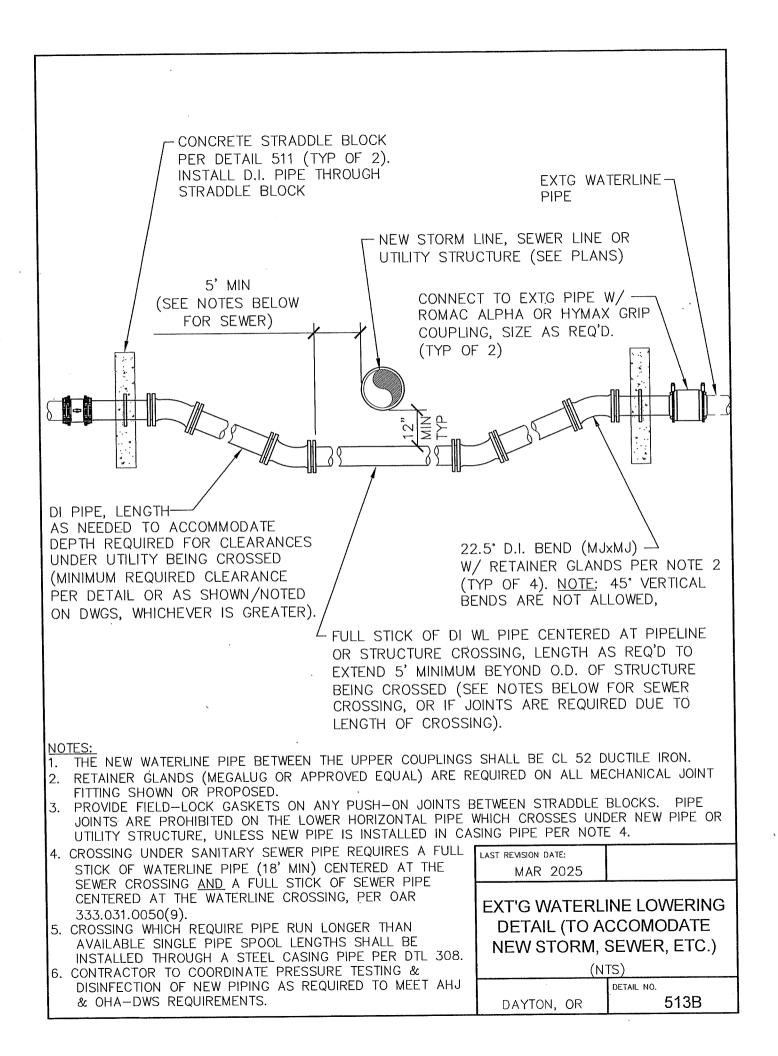
***	VERTICAL	BENDS	THAT	REQUIF	RE A	THRUST E	3LOCK
	OLUME EX	CEEDING	5 Cl	JBIC YA	ARDS	REQUIRE	SPECIAL
E	BLOCKING I	DETAILS.	SEE	PLANS	FOR	VOLUMES	SHOWN
l II	NSIDE HEA	VY LINE	IN T	ABLE.			

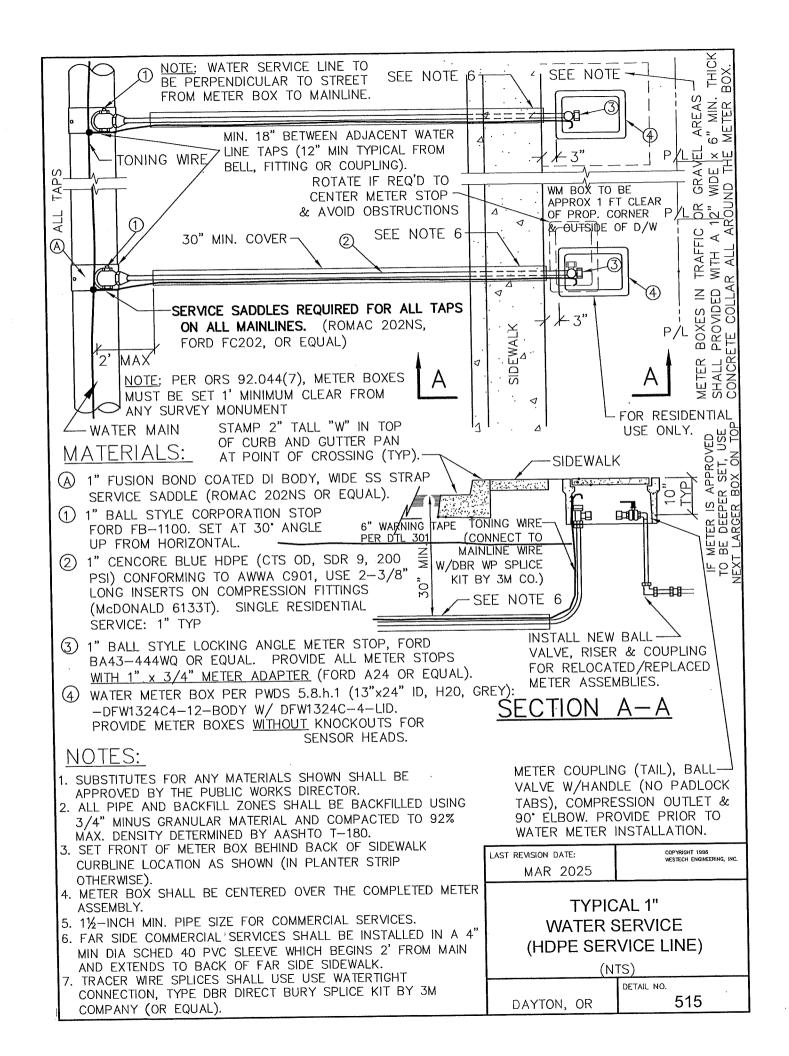
FITTING	DOD	EMBED-
FITTING	ROD	EMPED-
SIZE	SIZE	MENT
12" AND LESS	#6	30"
14" - 16"	#8	36"

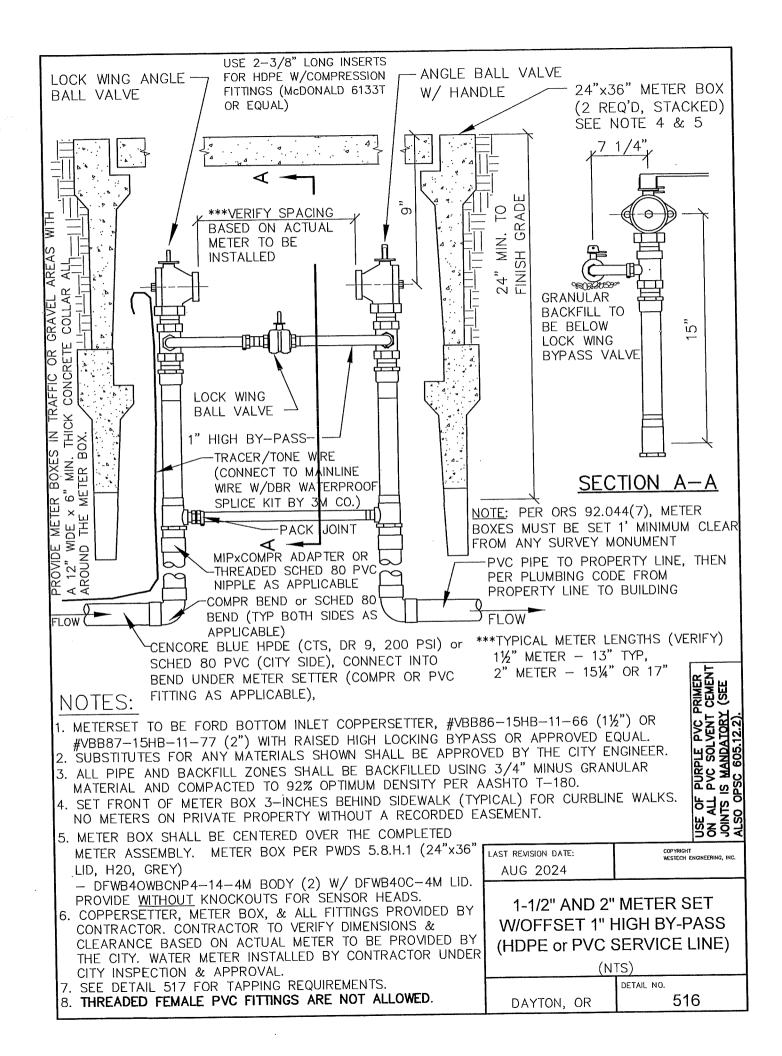
GRAVITY VERTICAL THRUST BLOCK FOR USE ONLY WHERE SPECIFICALLY APPROVED BY PUBLIC WORKS DIRECTOR AND CITY ENGINEER (TYPICALLY USE DTL 511 W/RETAINER GLANDS ON MJ JOINTS).

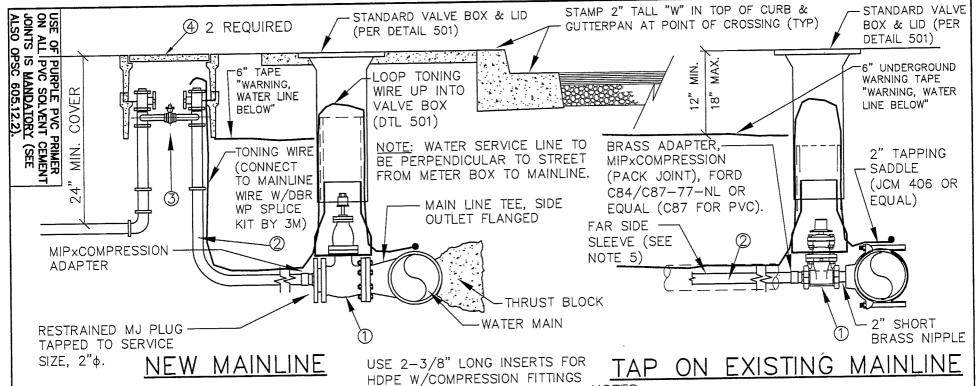
LAST REVISION DATE: OCT 2024	JO #			
VERTICLE THRUST BLOCKING (NTS)				
DAYTON, OR	DETAIL NO. 512			











(McDONALD 6133T OR EQUAL)

MATERIALS

LAST REVISION DATE:
DEC 2024

CONNECTION REQUIREMENTS
1-1/2" & 2" SERVICE (HDPE or
SCHED 80 PVC SERVICE LINE)

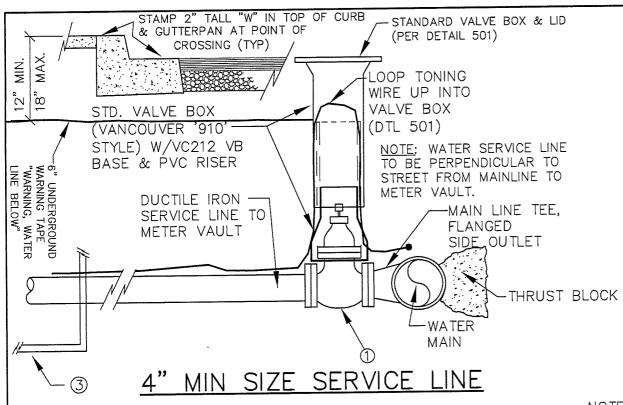
DAYTON

R

(NTS)

- 1 FLG X MJ RESILIENT WEDGE GATE VALVE PER AWWA C-509. EPOXY COATED PER AWWA C-550.
- (2) CENCORE BLUE HDPE (CTS, DR 9, 200 PSI, ≤2"¢)
 W/OUT JOINTS OF SCHEDULE 80 PVC PIPE & FITTINGS
 PER DETAIL 516 (30" MIN COVER TO METER). FEMALE
 THREADED PVC FITTINGS ARE NOT ALLOWED.
- (3) METER STOP ASSEMBLY W/BYPASS PER PUBLIC WORKS REQUIREMENTS. SEE DETAIL 516 FOR 1-1/2" &: 2" SERVICES.
- 4 METER BOX FOR 1-1/2" AND 2" SHALL BE PER DETAIL 516. USE TRAFFIC RATED VERSION OF BOX/LID FOR TRAFFIC AREAS.

- 1. SUBSTITUTES FOR ANY MATERIAL SHOWN SHALL BE APPROVED BY THE CITY ENGINEER.
- 2. ALL PIPE AND STRUCTURE ZONES SHALL BE BACKFILLED USING 3/4" MINUS GRANULAR MATERIAL AND COMPACTED TO 95% MAX DENSITY AS DETERMINED BY ASHTO T-180.
- 3. METER BOX SHALL BE CENTERED OVER THE COMPLETED METER AND FITTING ASSEMBLY.
- 4. CUSTOMER SHALL INSTALL AN APPROVED BACKFLOW PREVENTION DEVICE ON PRIVATE PROPERTY IMMEDIATELY DOWNSTREAM OF WATER METER IF REQUIRED BY PUBLIC WORKS.
- 5. UNLESS OTHERWISE APPROVED BY THE PUBLIC WORKS DIRECTOR ON A CASE—BY—CASE BASIS, FAR SIDE COMMERCIAL SERVICES SHALL BE INSTALLED IN A 4" MIN DIA SCHED 40 PVC SLEEVE WHICH BEGINS 6" FROM MAINLINE VALVE & EXTENDS TO EDGE OF FAR SIDE METER BOX.
- 6. <u>COLLAR</u>: METER BOXES IN TRAFFIC OR GRAVEL AREAS SHALL PROVIDED WITH A 12" WIDE x 6" MIN. THICK CONCRETE COLLAR ALL AROUND THE METER BOX.

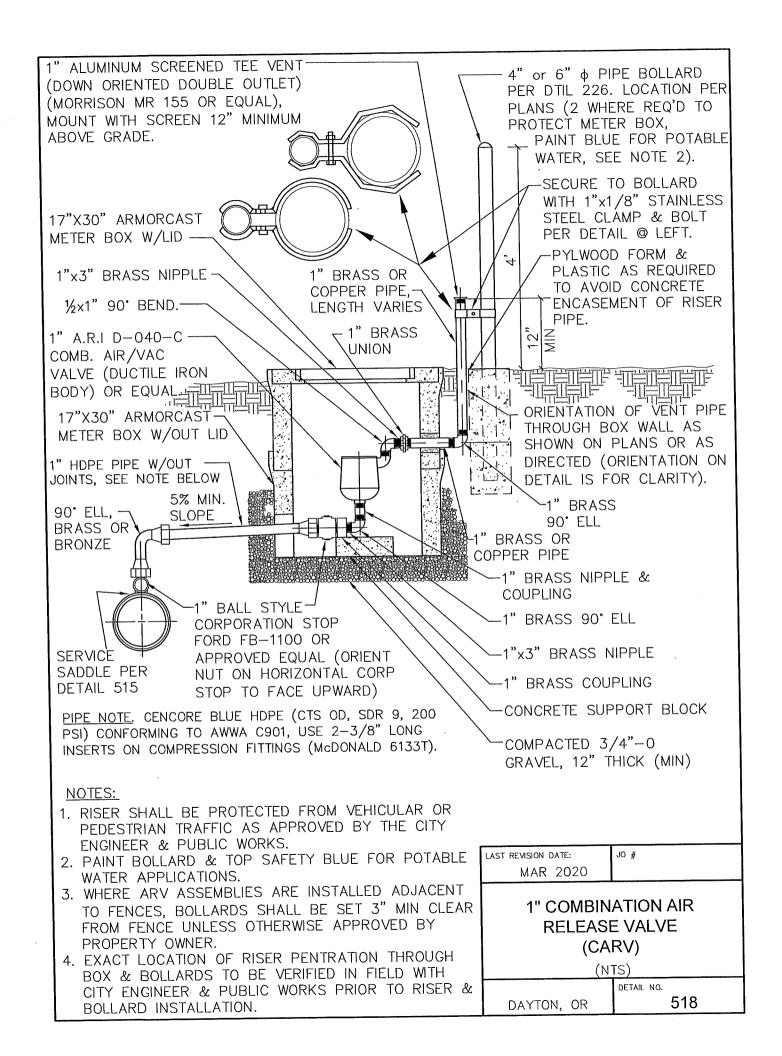


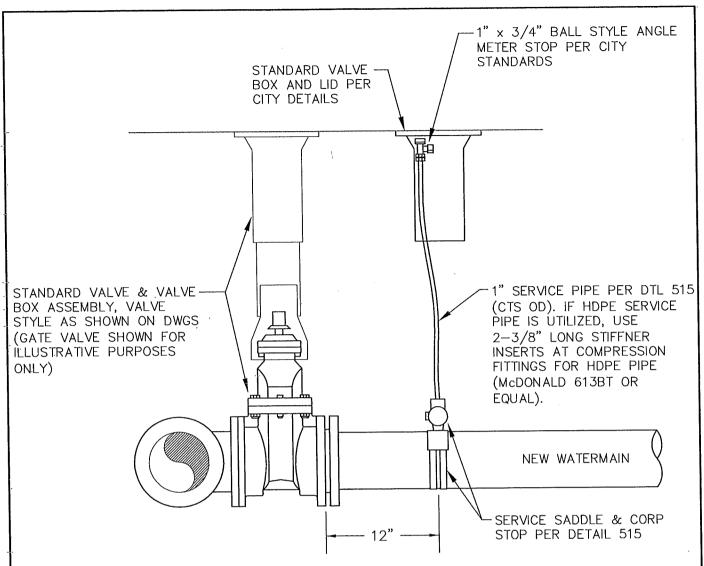
CONNECTION REQUIREMENTS, 3" AND LARGER METER (NTS) DAYTON, OR COPRIGHT 1995 WESTECH DISMEDING, INC. DETAIL NO. DETAIL NO. DETAIL NO. 517B

MATERIALS

- 1) FLG X MJ RESILIENT WEDGE GATE VALVE PER AWWA C-509. 4" MIN OR SERVICE SIZE, WHICHEVER IS LARGER. EPOXY COATED PER AWWA C-550.
- ② SERVICE PIPE TO BE CL 52 DI PIPE TO METER VAULT.
- 3 SEE DETAILS 523-526 FOR CONFIGURATION AT METER VAULT.

- 1. SUBSTITUTES FOR ANY MATERIAL SHOWN SHALL BE APPROVED BY THE CITY ENGINEER.
- 2. ALL PIPE AND STRUCTURE ZONES SHALL BE BACKFILLED USING 3/4" MINUS GRANULAR MATERIAL AND COMPACTED TO 95% MAX DENSITY AS DETERMINED BY ASHTO T-180.
- 3. METER BOX SHALL BE CENTERED OVER THE COMPLETED METER AND FITTING ASSEMBLY.
- 4. CUSTOMER SHALL INSTALL AN APPROVED BACKFLOW PREVENTION DEVICE ON PRIVATE PROPERTY IMMEDIATELY DOWNSTREAM OF WATER METER IF REQUIRED BY PUBLIC WORKS.
- 5. FOR EXISTING MAINLINES, INSTALL APPLICABLE SIZE HOT TAP PER DETAIL 505.





NOTES:

- 1. DISTANCE FROM WATERLINE VALVE TO CHLORINE TAP SHALL BE 12" UNLESS OTHERWISE DIRECTED OR APPROVED IN WRITING BY THE PUBLIC WORKS DIRECTOR OR DESIGNEE.
- 2. THE VALVE BOX SHOWN (OVER THE CHLORINATION METER STOP) IS <u>NOT</u> REQUIRED IF THE CHLORINATION LINE TERMINATES WITH THE METER STOP LOCATED BEHIND THE CURB.

 IF THE CHLORINATION LINE TERMINATES BEHIND THE CURB, THE METER STOP SHALL BE SET 6"
 ABOVE FINISH GRADE AND CLEARLY MARKED WITH ORANGE FLAGGING AND A TRAFFIC CONE.

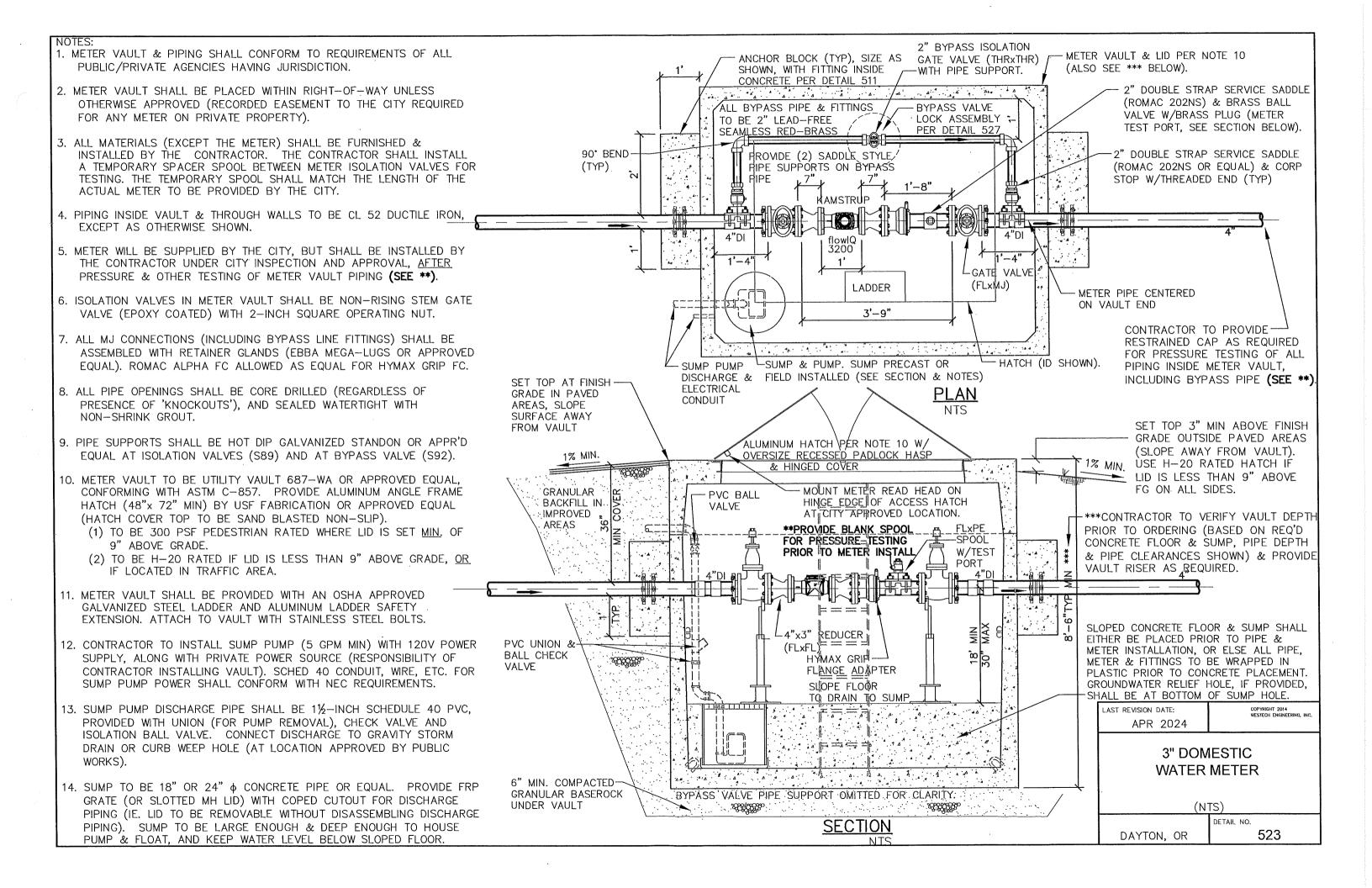
3. UNLESS OTHERWISE DIRECTED BY THE CITY, THE CHLORINATION PROCESS SHALL BE COMPLETED BY THE CONTRACTOR PER CITY STANDARDS, UNDER THE OBSERVATION OF PUBLIC WORKS STAFF.

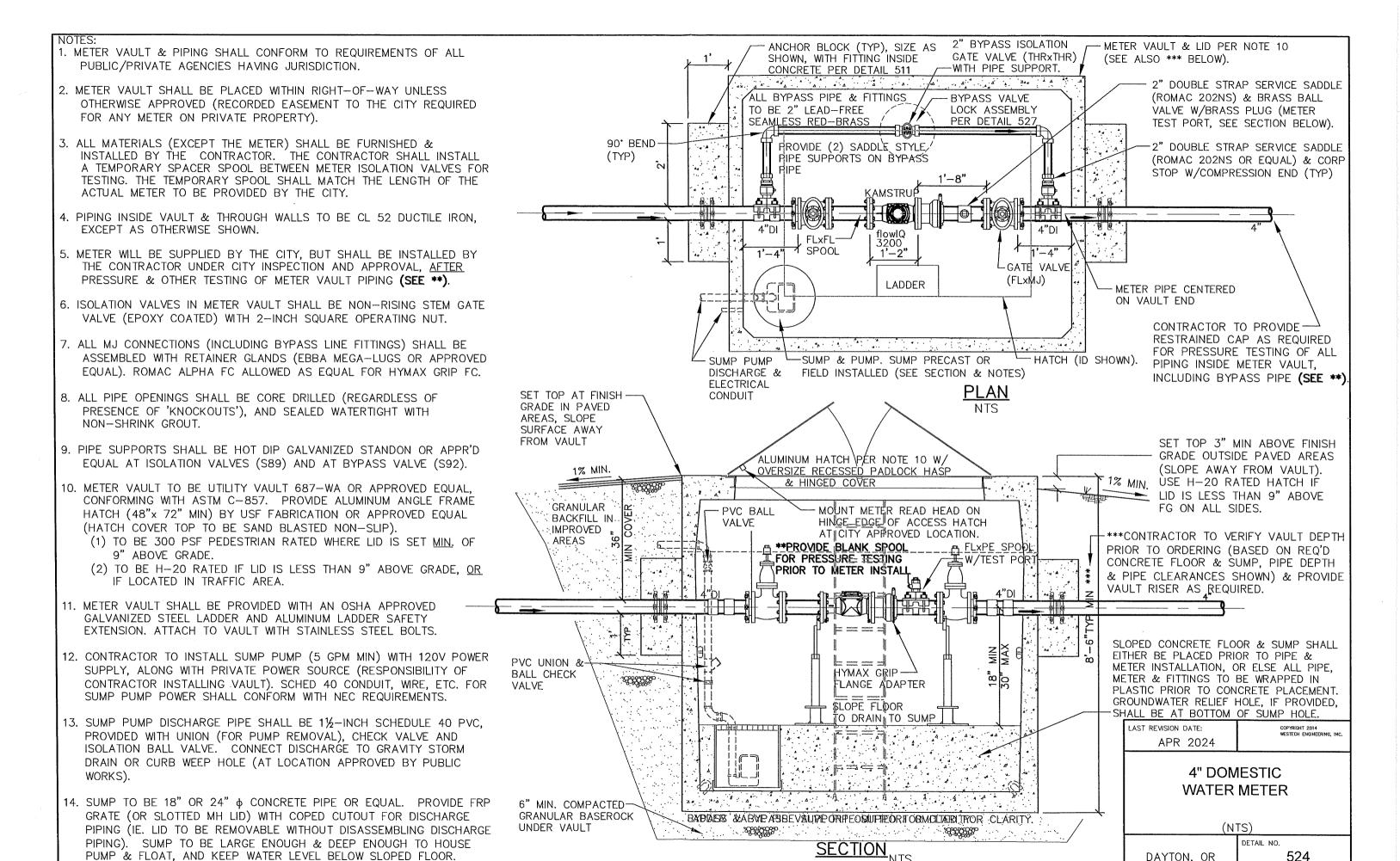
- 4. UNLESS OTHERWISE DIRECTED BY THE CITY, THE CONTRACTOR SHALL NOT REMOVE THE CHLORINATION ASSEMBLY UNTIL AFTER RECEIVING NOTICE OF NEGATIVE BACTERIOLOGICAL TEST RESULTS AND AFTER APPROVAL FROM PUBLIC WORKS. CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIAL, EXCAVATION, BACKFILL, FINAL SURFACE RESTORATION, ETC.
- 5. UNLESS OTHERWISE APPROVED OR REQUIRED (IN WRITING) BY THE PUBLIC WORKS DIRECTOR, ALL EXTRA PIPE & FITTINGS ASSOCIATED WITH THE CHLORINATION TAP ASSEMBLY SHALL BE REMOVED AFTER THE NEW WATERLINE IS PLACED IN SERVICE. THE CHLORINATION TAP SHALL BE CAPPED WITH A BRASS CAP ON THE CORP STOP (TO AVOID DEPRESSURIZING THE MAINLINE AFTER DISINFECTION). EACH CAPPED CORP STOP SHALL BE WRAPPED IN PLASTIC PRIOR TO BACKFILLING.
- 6. THE LOCATION OF EACH CAPPED CHLORINATION CORP STOP SHALL BE SHOWN ON THE CONTRACTOR'S RECORD DRAWINGS AND ALSO ON THE FINAL AS—BUILTS.

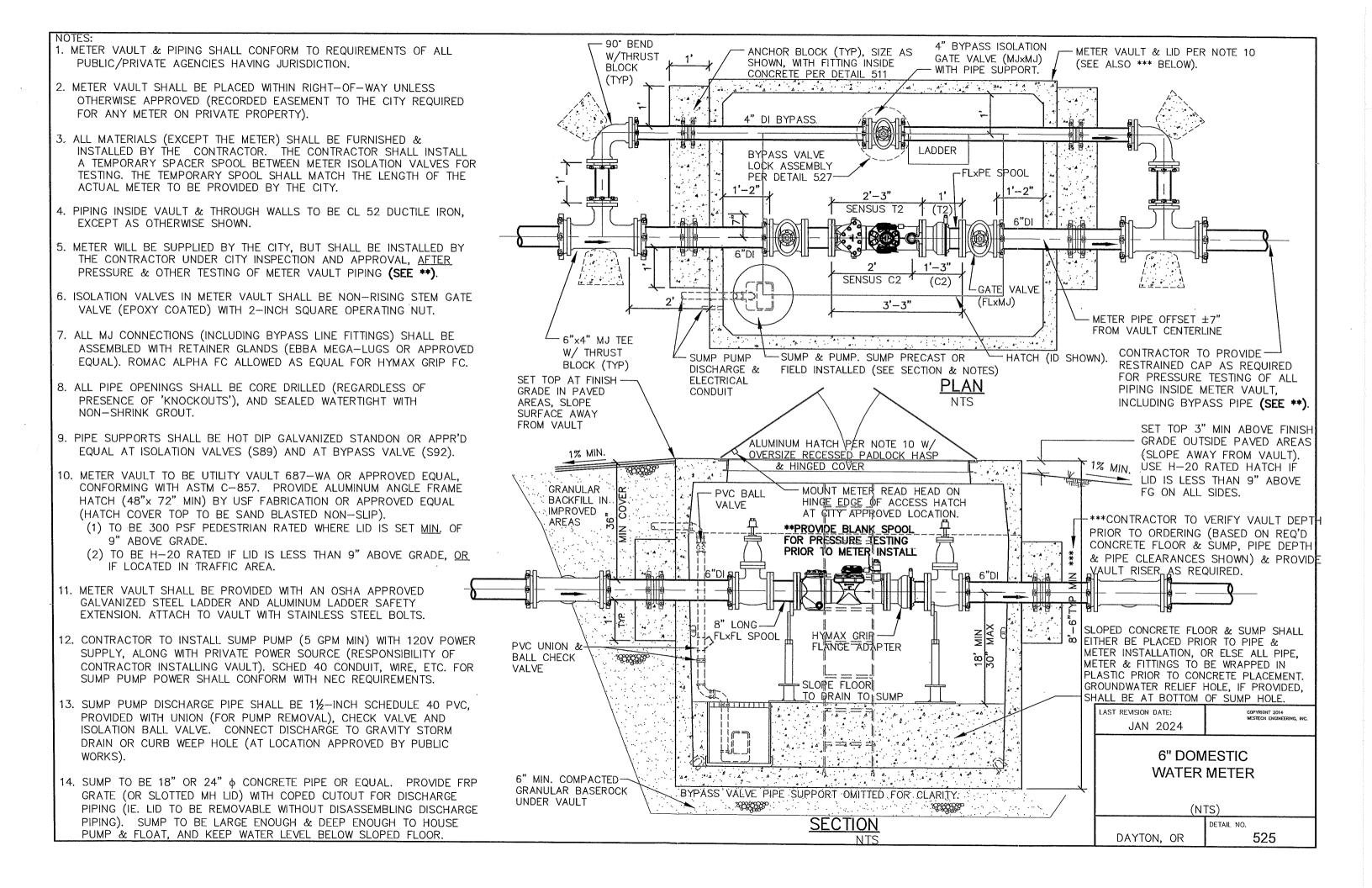
LAST REVISION DATE:	JO #			
JAN 2024				
POTABLE WATERLINE CHLORINATION TAP ASSEMBLY				
(N	TS)			
	DETAIL NO.			

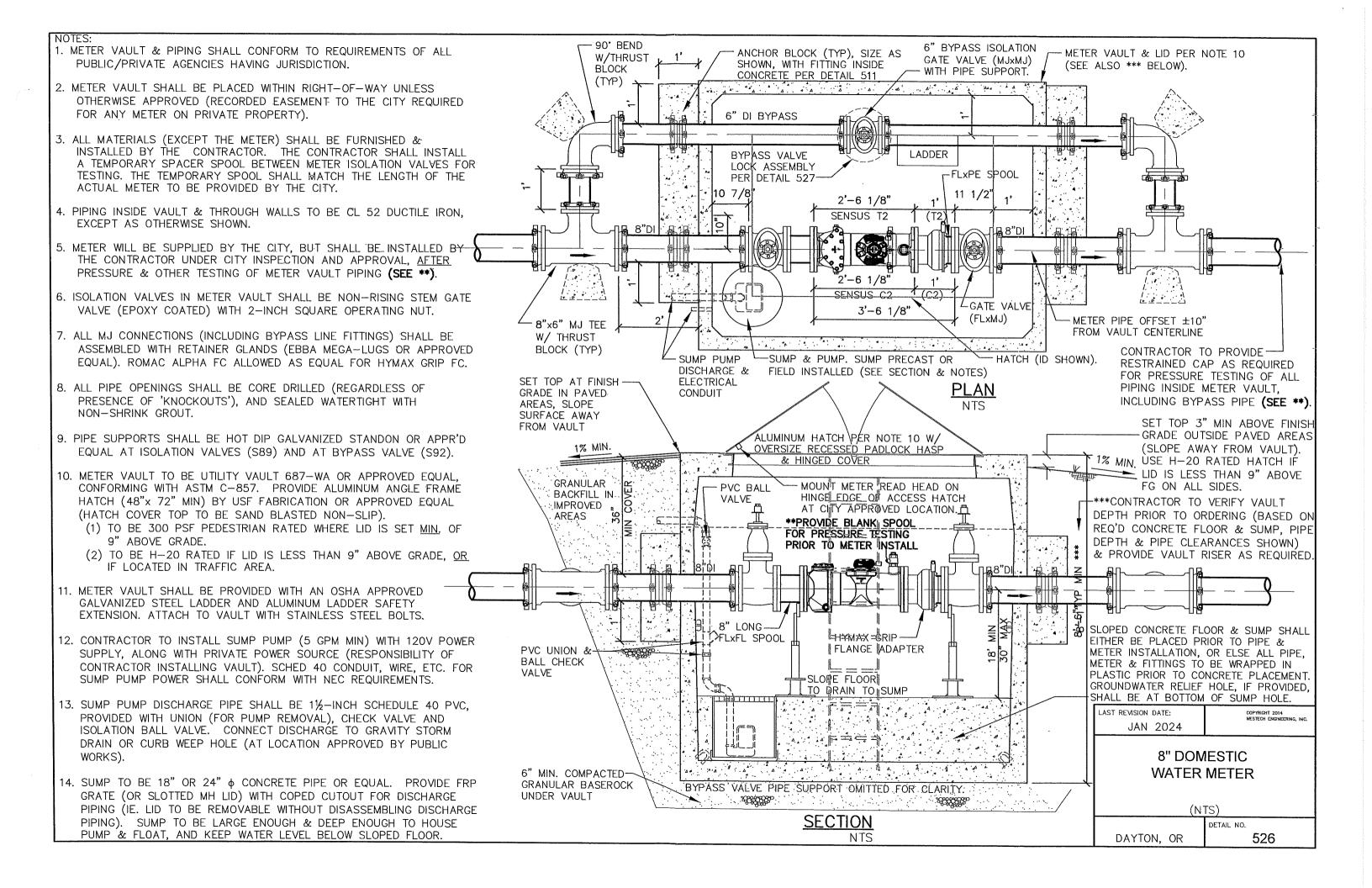
DAYTON, OR

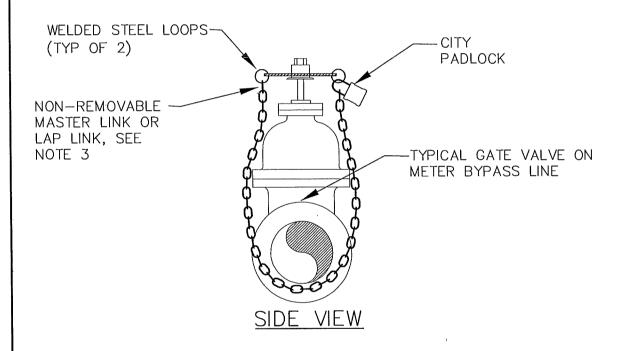
519





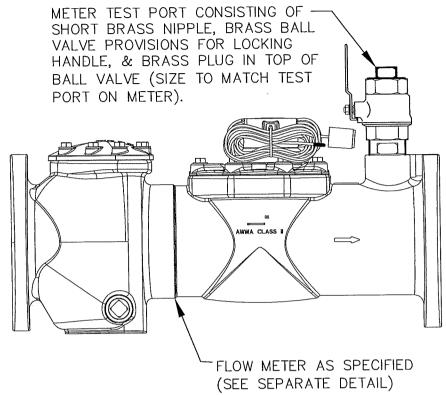






- 1. UNLESS OTHERWISE REQUIRED BY PUBLIC WORKS, PROVIDE ONE VALVE LOCK PLATE ASSEMBLY PER METER VAULT.
- 2. VALVE LOCK PLATE ASSEMBLY TO BE HOT DIP GALVANIZED AFTER FABRICATION.
- 3. INSTALL CHAIN AFTER VALVE LOCK PLATE ASSEMBLY IS GALVANIZED. CONNECT CHAIN TO STEEL LOOP WITH NON-REMOVABLE MASTER LINK OR LAP LINK (STAINLESS STEEL OR GALVANIZED STEEL).

	LAST REVISION DATE:	JO #				
	JUNE 2025					
1						
	WATER METER VAULT,					
	BYPASS VALVE LOCK					
	PLATE ASSEMBLY					
	(NTS)					
	714	DETAIL NO.				
	DAYTON, OR	527				



OPTION A (FOR METER W/TEST PORT TAP) FOR METERS WITHOUT AN INTEGRAL TEST PORT TAP, SEE NOTE 2 BELOW.

NOTES:

- 1. UNLESS OTHERWISE APPROVED IN WRITING BY THE PUBLIC WORKS DIRECTOR AND CITY ENGINEER, ALL METERS 3" & LARGER SHALL BE PROVIDED WITH A TEST PORT ASSEMBLY IN THE VAULT (DOWNSTREAM OF THE METER) CONSISTING OF A BRASS NIPPLE, BALL VALVE AND BRASS PLUG AS SHOWN ABOVE.
- 2. FOR METERS WITHOUT A BUILT—IN TEST PORT TAP, PROVIDE A 2" TEST PORT INSIDE THE VAULT (DOWNSTREAM OF METER) ON A 2" DOUBLE STRAP SERVICE SADDLE (ROMAC 202NS OR EQUAL), WITH BALL VALVE & BRASS PLUG AS SHOWN ABOVE.
- 3. METER TESTING. THE CONTRACTOR SHALL PROVIDE ALL FITTINGS & HOSES NECESSARY TO TEST FLOW WATER THROUGH THE METER AFTER INSTALLATION, IN ORDER TO DEMONSTRATE PROPER OPERATION OF THE METER WITH PUBLIC WORKS STAFF PRESENT (CONTRACTOR SHALL COORDINATE WITH METER REPRESENTATIVE AS NECESSARY FOR SUCH TESTING & DEMONSTRATION OF PROPER OPERATION).

LAST REVISION DATE:

APR 2024

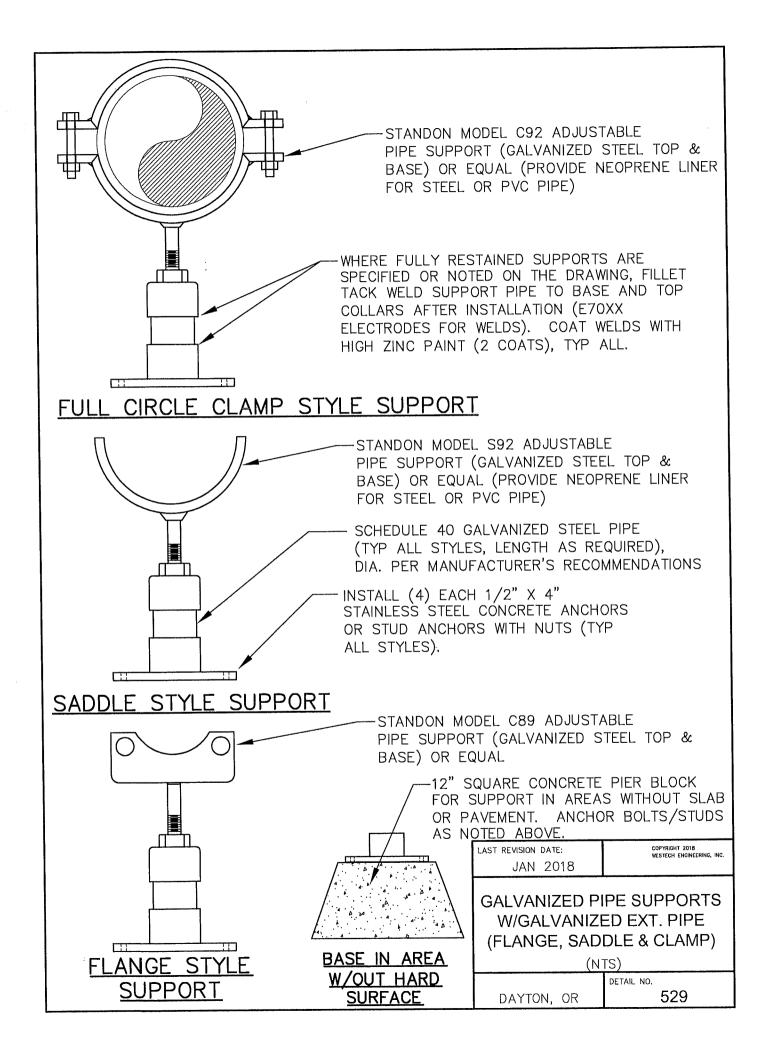
WATER METER TEST PORT ASSEMBLY

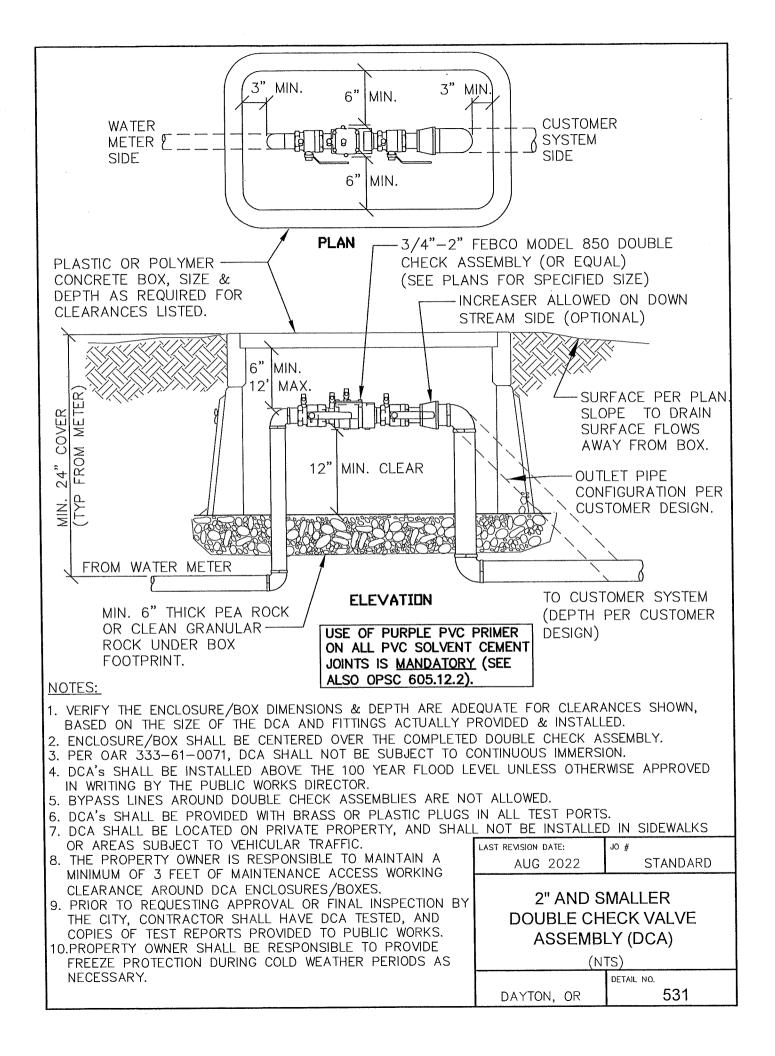
(NTS)

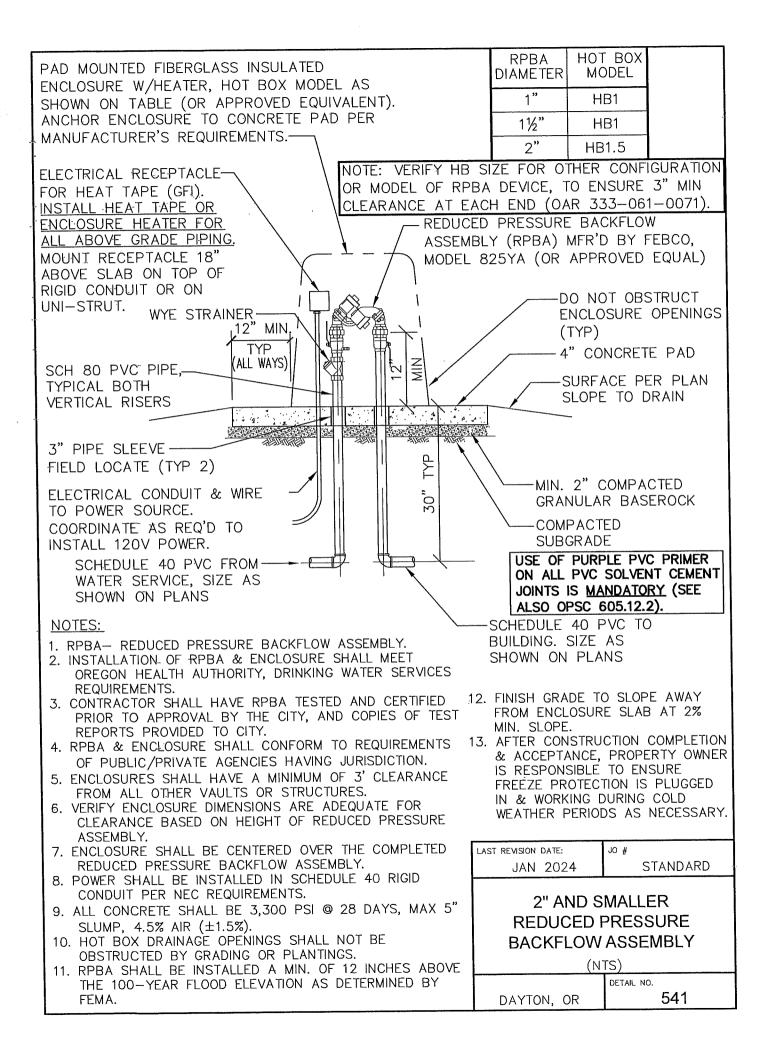
DETAIL NO.

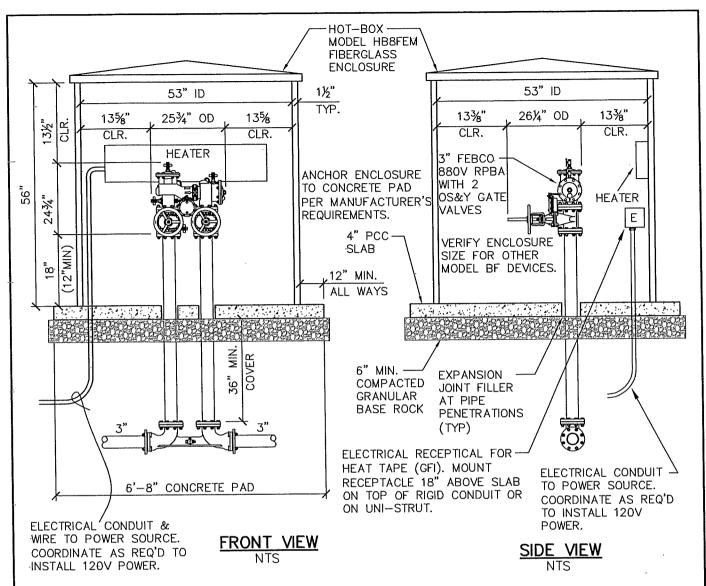
DAYTON, OR

528





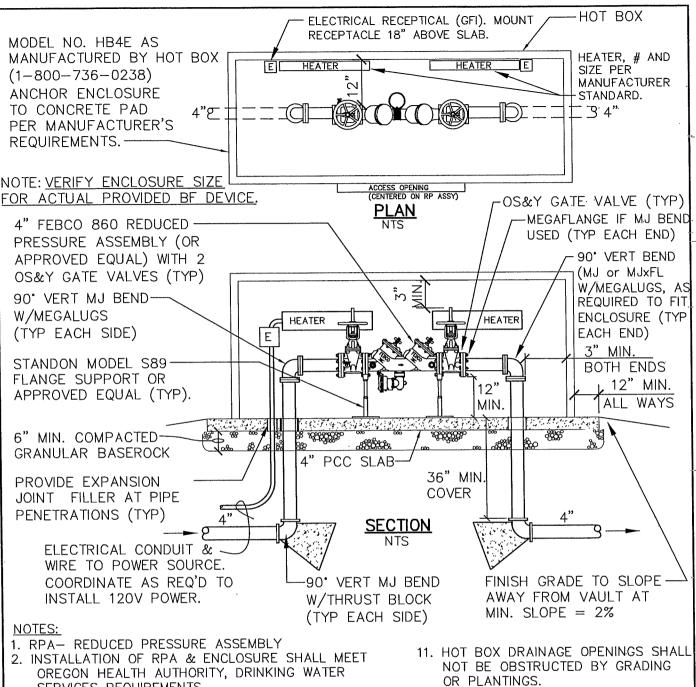




- 1. RPBA— REDUCED PRESSURE BACKFLOW ASSEMBLY.
- 2. INSTALLATION OF RPBA & ENCLOSURE SHALL MEET OREGON HEALTH AUTHORITY, DRINKING WATER SERVICES REQUIREMENTS.
- 3. CONTRACTOR SHALL HAVE RPBA TESTED AND CERTIFIED PRIOR TO APPROVAL BY THE CITY, AND COPIES OF TEST REPORTS PROVIDED TO CITY.
- 4. RPBA & ENCLOSURE SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
- 5. ENCLOSURES SHALL HAVE A MINIMUM OF 3' CLEARANCE FROM ALL OTHER VAULTS OR STRUCTURES.
- 6. VERIFY ENCLOSURE DIMENSIONS ARE ADEQUATE FOR CLEARANCE BASED ON HEIGHT OF REDUCED PRESSURE ASSEMBLY.
- 7. ENCLOSURE SHALL BE CENTERED OVER THE COMPLETED REDUCED PRESSURE BACKFLOW ASSEMBLY.
- 8. POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
- 9. ALL CONCRETE SHALL BE 3,300 PSI @ 28 DAYS, MAX 5" SLUMP, 4.5% AIR (±1.5%).
- 10. HOT BOX DRAINAGE OPENINGS SHALL NOT BE OBSTRUCTED BY GRADING OR PLANTINGS.
- 11. RPBA SHALL BE INSTALLED A MIN. OF 12 INCHES ABOVE THE 100—YEAR FLOOD ELEVATION AS DETERMINED BY FEMA.

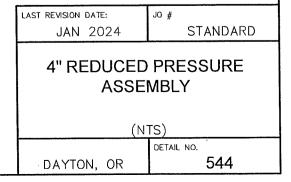
- 12. FINISH GRADE TO SLOPE AWAY FROM ENCLOSURE SLAB AT 2% MIN. SLOPE.
- 13. RISER PIPES & ABOVE GRADE PIPING SHALL BE DUCTILE IRON (CL 52 MIN).

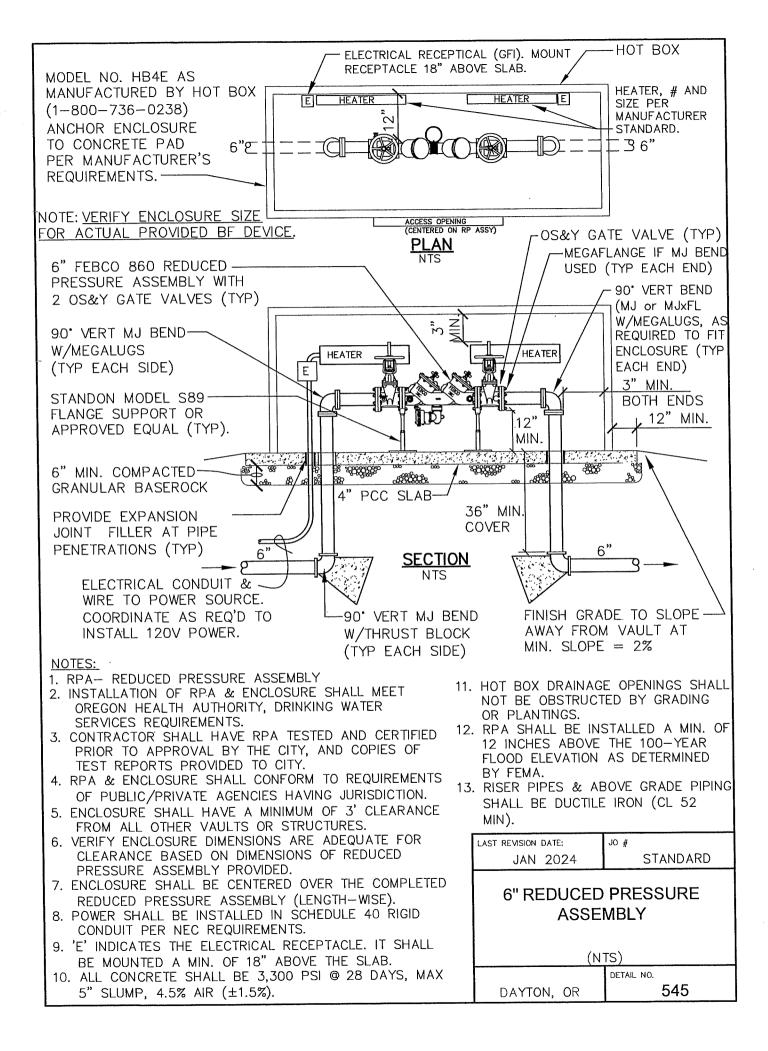
LAST REVISION DATE: JAN 2024	JO #
3" REDUCED ASSEI	
(N	TS)
DAYTON, OR	DETAIL NO. 543

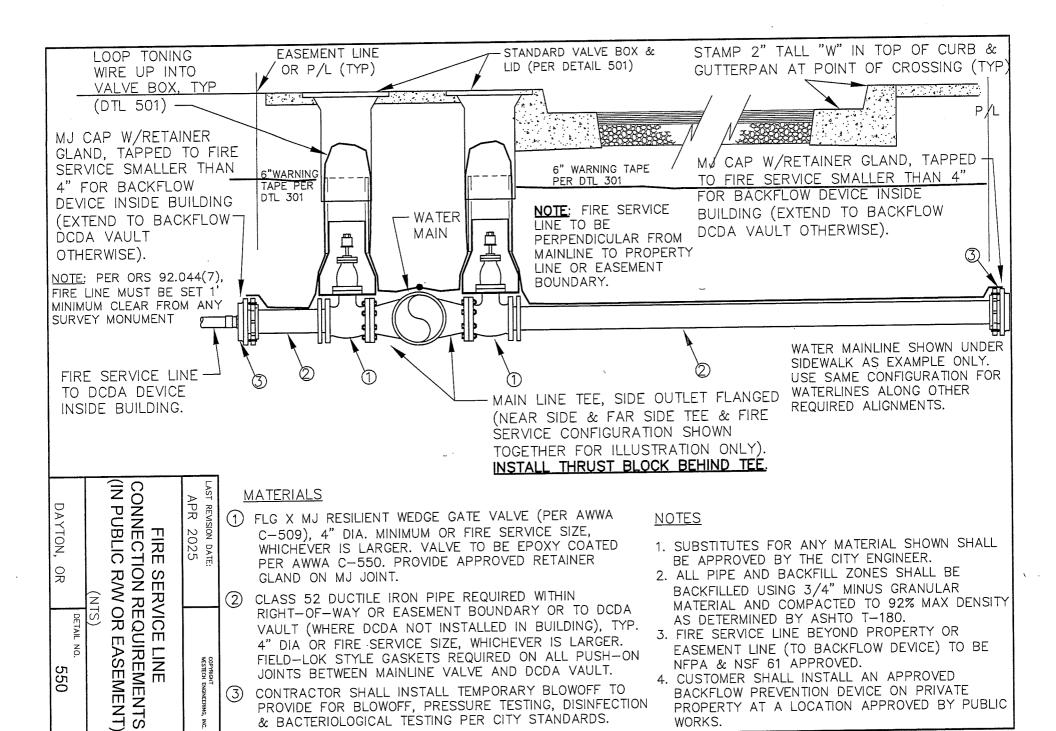


- SERVICES REQUIREMENTS.
- 3. CONTRACTOR SHALL HAVE RPA TESTED AND CERTIFIED PRIOR TO APPROVAL BY THE CITY, AND COPIES OF TEST REPORTS PROVIDED TO CITY.
- 4. RPA & ENCLOSURE SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
- 5. ENCLOSURE SHALL HAVE A MINIMUM OF 3' CLEARANCE
- FROM ALL OTHER VAULTS OR STRUCTURES.
 6. VERIFY ENCLOSURE DIMENSIONS ARE ADEQUATE FOR CLEARANCE BASED ON DIMENSIONS OF REDUCED PRESSURE ASSEMBLY PROVIDED.
- 7. ENCLOSURE SHALL BE CENTERED OVER THE COMPLETED REDUCED PRESSURE ASSEMBLY (LENGTH-WISE).
- 8. POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
- 9. 'E' INDICATES THE ELECTRICAL RECEPTACLE. IT SHALL BE MOUNTED A MIN. OF 18" ABOVE THE SLAB.
- 10. ALL CONCRETE SHALL BE 3,300 PSI @ 28 DAYS, MAX 5" SLUMP, 4.5% AIR (±1.5%).

- 12. RPA SHALL BE INSTALLED A MIN. OF 12 INCHES ABOVE THE 100-YEAR FLOOD ELEVATION AS DETERMINED BY FEMA.
- 13. RISER PIPES & ABOVE GRADE PIPING SHALL BE DUCTILE IRON (CL 52 MIN).



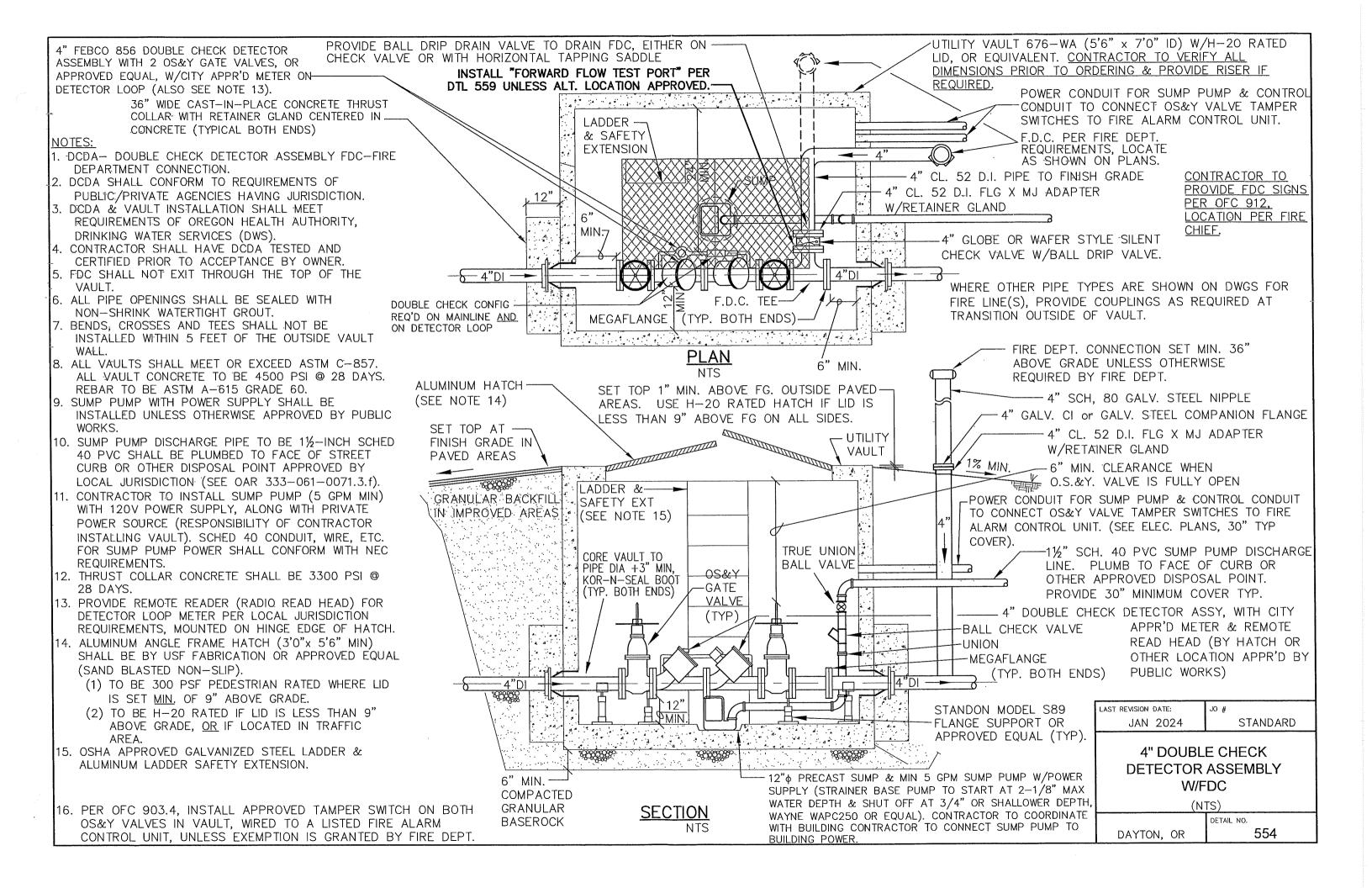


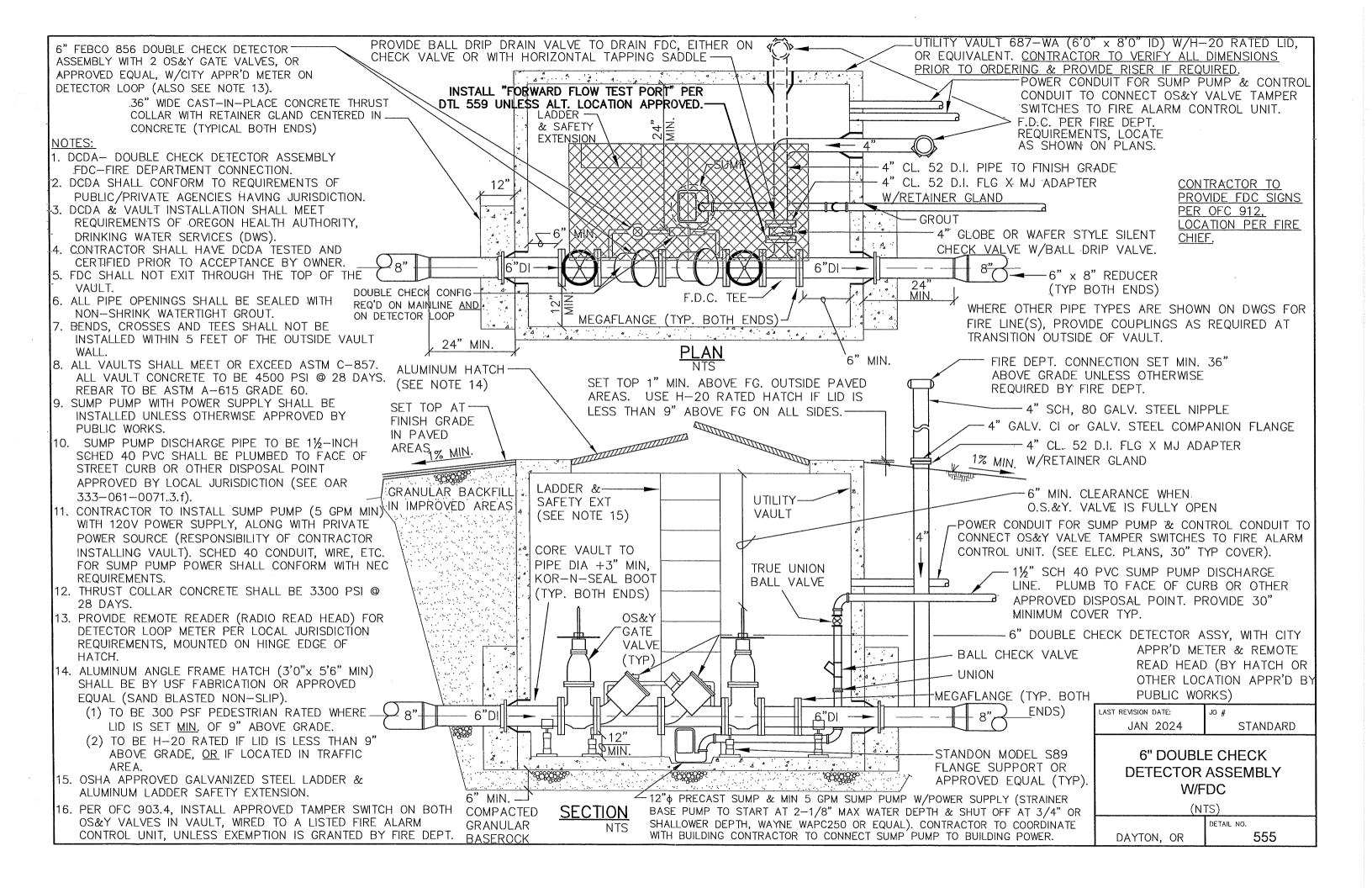


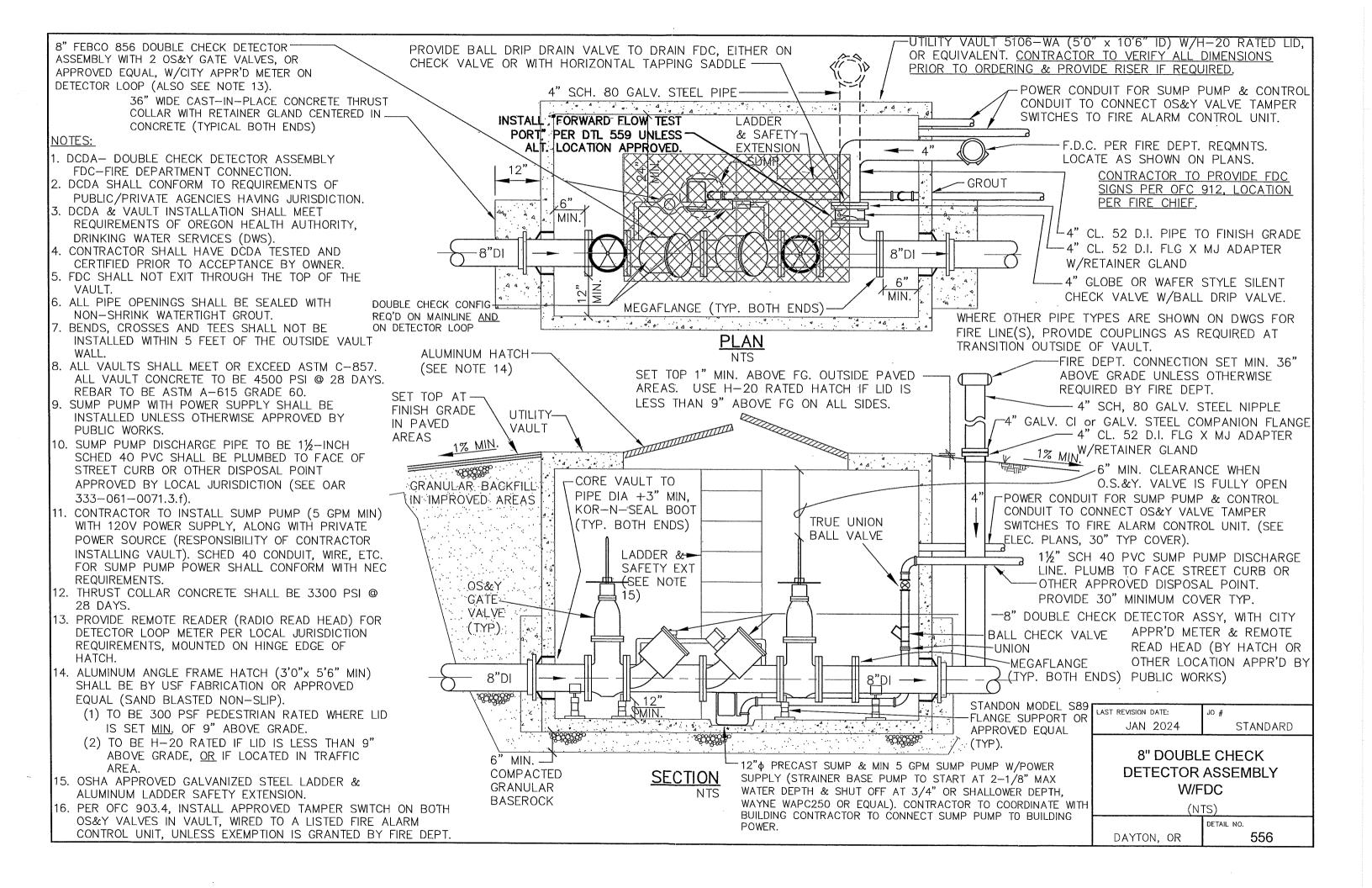
WORKS.

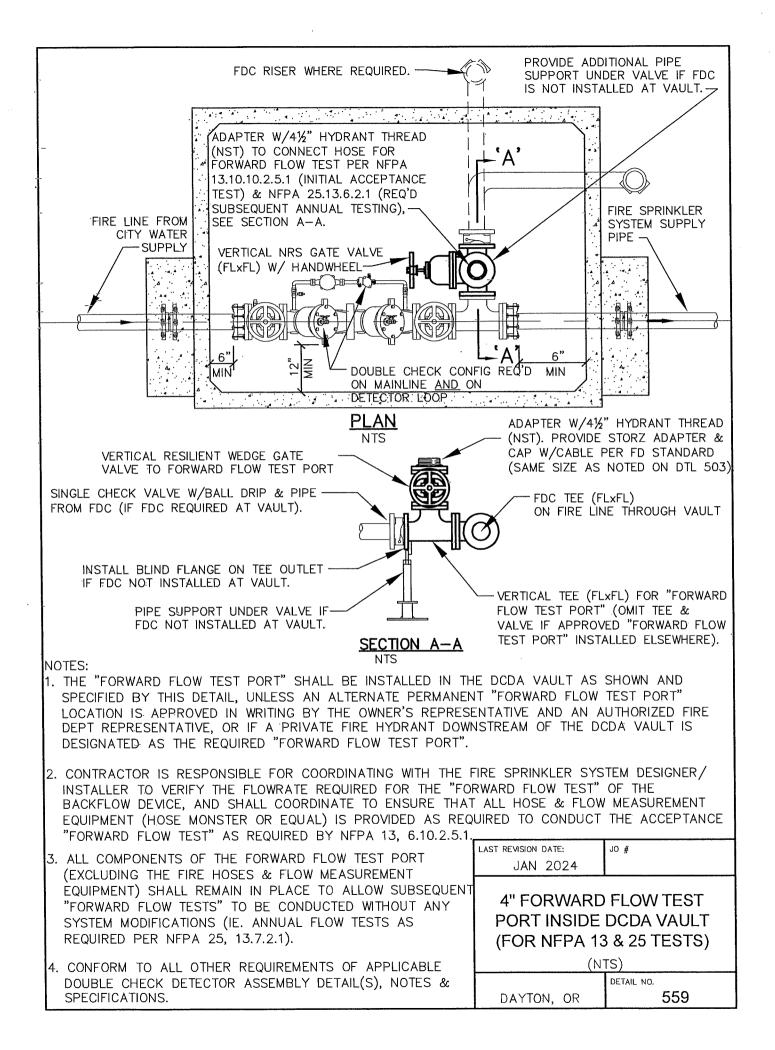
& BACTERIOLOGICAL TESTING PER CITY STANDARDS.

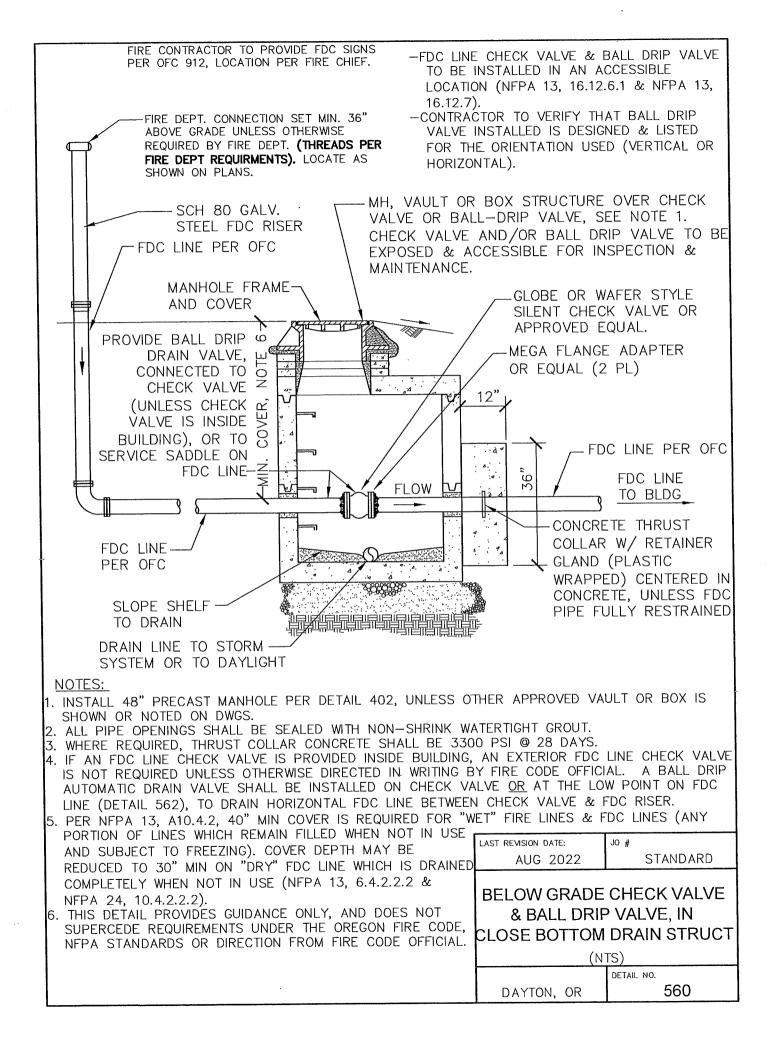
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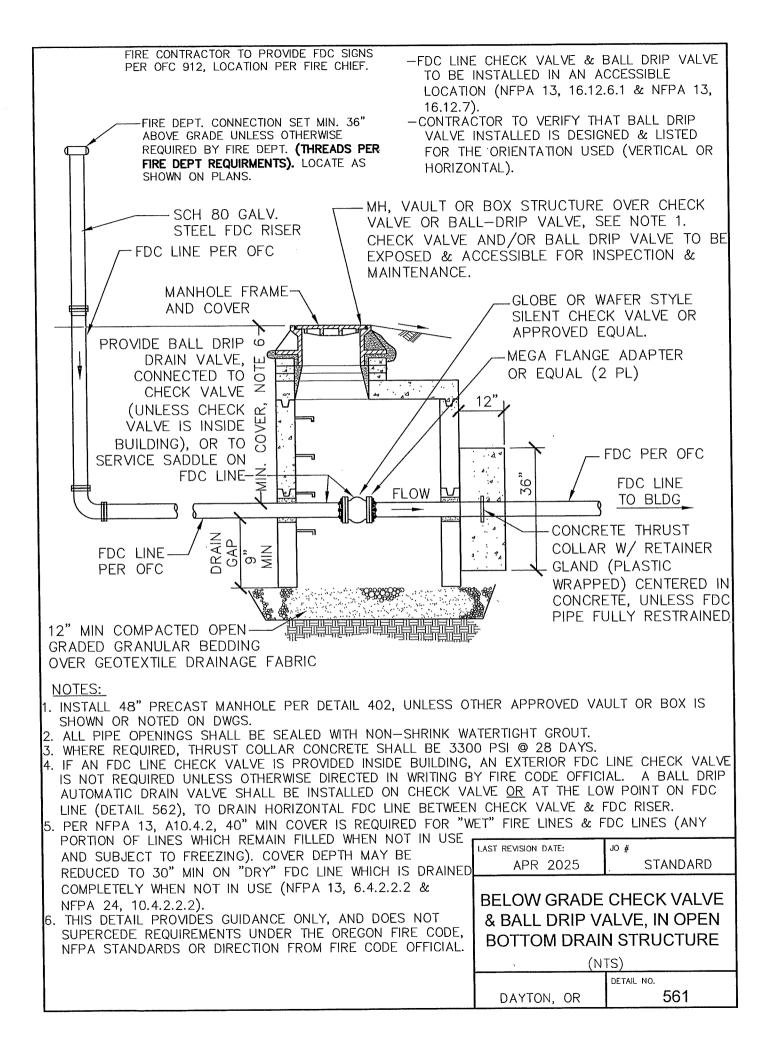


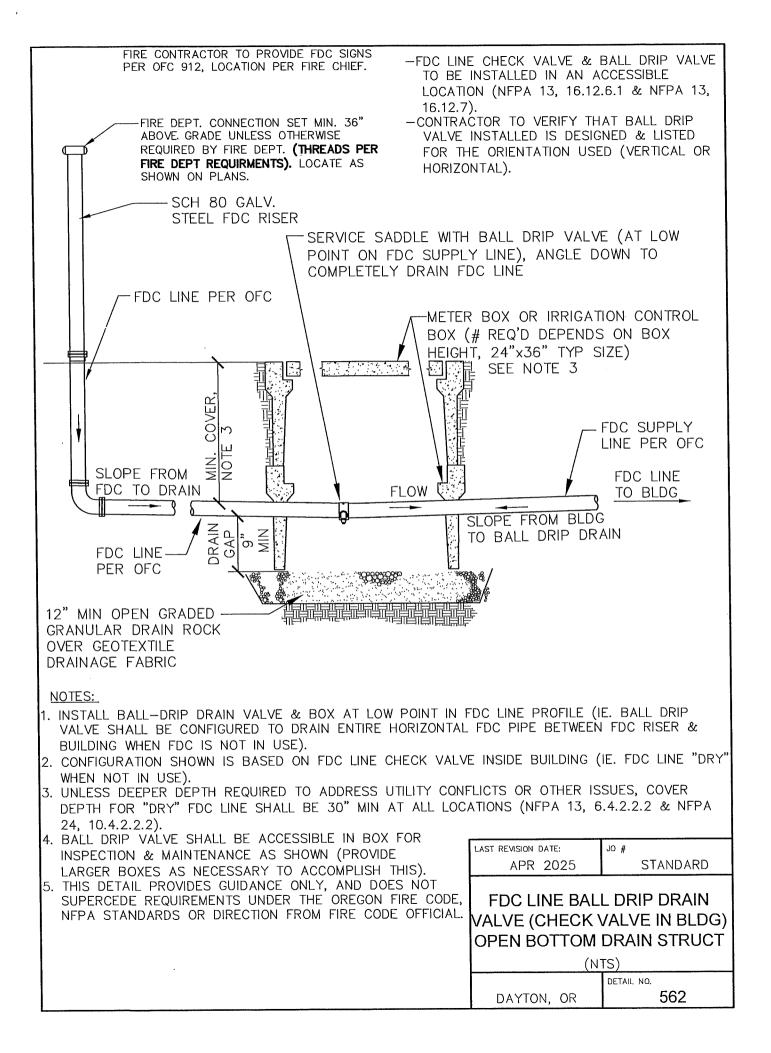


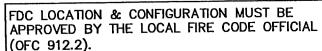












SINGLE CHECK VALVE INSIDE BUILDING (SEE FIRE SPRINKLER DESIGN).

THREADED

GALV BEND

SIGNS. ETC.: FIRE CONTRACTOR TO PROVIDE FDC SIGNS PER OFC 912, LOCATION PER FIRE CHIEF. PROVIDE CURB PAINTING IN FRONT OF FDC IF REQUIRED BY FIRE CODE OFFICIAL.

-FDC LINE CHECK VALVE & BALL DRIP VALVE TO BE INSTALLED IN AN ACCESSIBLE LOCATION (NFPA 13, 16.12.6.1 & NFPA 13, 16.12.7).

-CONTRACTOR TO VERIFY THAT BALL DRIP VALVE INSTALLED IS DESIGNED & LISTED FOR THE ORIENTATION USED (VERTICAL OR HORIZONTAL).

FIRE DEPT. CONNECTION SET MIN. 36" ABOVE GRADE UNLESS OTHERWISE REQUIRED BY FIRE DEPT. (COORDINATE WITH FIRE DEPT FOR SIZE & STYLE OF FDC CONNECTION). VERIFY FDC HEIGHT & ACCESSIBLE LOCATION AS APPROVED BY FIRE CODE OFFICIAL.

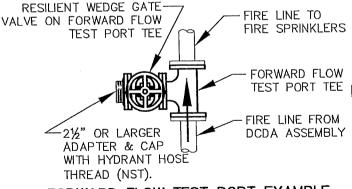
SCH 80 GALV. STEEL FDC PIPE THROUGH BUILDING WALL. BUILDING EXERIOR WALL

(HEATED INSIDE). PROVIDE BALL DRIP DRAIN VALVE (SEE BALL DRIP NOTE 1 BELOW).

FDC SUPPLY LINE TO SPRINKLER SYSTEM PER OFC & NFPA STANDARDS (CONNECT ON SPRINKLER SIDE OF DCDA)

FORWARD FLOW TEST DRAIN NOTES:

1. IF THE FORWARD FLOW TEST PORT IS INSTALLED INSIDE A BUILDING, DRAINS ADEQUATE TO HANDLE THE FULL TEST FLOWS SHALL BE PROVIDED, UNLESS PROVISIONS ARE INCLUDED TO DIRECT THE TEST FLOWS TO THE EXTERIOR OF THE BUILDING IN A LOCATION WHICH WILL NOT CAUSE DAMAGE TO PUBLIC OR PRIVATE PROPERTY



FORWARD FLOW TEST PORT EXAMPLE

BALL DRIP NOTE: INSTALL BALL-DRIP DRAIN VALVE AT LOW POINT IN FDC LINE PROFILE (UNLESS FDC LINE IS SLOPED TO DRAIN OUT COMPLETÈLY FROM CHECK VALVE TO BUILDING EXTERIOR WHEN FDC IS NOT IN USE).

GENERAL OFC & NFPA NOTE:

THIS DETAIL PROVIDES GUIDANCE ONLY, AND DOES NOT SUPERCEDE REQUIREMENTS UNDER THE OREGON FIRE CODE (OFC), NFPA STANDARDS OR DIRECTION FROM LOCAL FIRE CODÉ OFFICIAL OR FIRE CHIEF.

FORWARD FLOW TEST PORT NOTES:

- 1. A PERMANENT VALVED "FORWARD FLOW TEST PORT" SHALL BE INSTALLED ON SPRINKLER SIDE OF DCDA ASSEMBLY, AT A LOCATION AS APPROVED IN WRITING BY THE FIRE CODE OFFICIAL, UNLESS A PRIVATE FIRE HYDRANT DOWNSTREAM OF THE DCDA IS DESIGNATED AS THE REQUIRED "FORWARD" FLOW TEST PORT".
- 2. CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH THE FIRE SPRINKLER SYSTEM DESIGNER/ INSTALLER TO VERIFY THE FLOWRATE REQUIRED FOR THE "FORWARD FLOW TEST" OF THE BACKFLOW DEVICE, AND SHALL COORDINATE TO ENSURE THAT ALL HOSE & FLOW MEASUREMENT EQUIPMENT (HOSE MONSTER OR EQUAL) IS PROVIDED AS REQUIRED TO CONDUCT THE ACCEPTANCE "FORWARD FLOW TEST" AS REQUIRED BY NFPA 13, 6.10.2.5.1.
- 3. ALL COMPONENTS OF THE FORWARD FLOW TEST PORT (EXCLUDING THE FIRE HOSES & FLOW MEASUREMENT EQUIPMENT) SHALL REMAIN IN PLACE TO ALLOW SUBSEQUENT "FORWARD FLOW TESTS" TO BE CONDUCTED WITHOUT ANY SYSTEM MODIFICATIONS (IE. ANNUAL FLOW TESTS AS REQUIRED PER NFPA 25, 13.7.2.1).

TEST PORT TEE DCDA & DETECTOR LOOP METER NOTES:

CONFORM TO ALL OTHER REQUIREMENTS OF FIRE LINE FROM APPLICABLE DOUBLE CHECK DETECTOR ASSEMBLY DCDA ASSEMBLY DETAIL(S), NOTES & SPECS ON CITY DETAILS OR STANDARDS (INCLUDING INSTALLATION OF A CITY APPROVED WATER METER & DOUBLE CHECK FOR DCDA DETECTOR LOOP, AT AN ACCESSIBLE LOCATION ACCEPTABLE TO PUBLIC WORKS).

JO # LAST REVISION DATE: STANDARD SEPT 2024 SAMPLE & NOTES, FDC ON

BLDG EXTERIOR, FORWARD FLOW TEST PORT, DCDA, ETC.

(NTS) DETAIL NO. 563 DAYTON, OR

WATERLINE PRESSURE TEST REPORT

Project Location:	Project Name:	Date:			
Inspector: (Print)	Waterline to be tested. From Station:	To Station:			
Verify that all in-line valves, including hydrant mainline valves, are open? Yes / No					
Verify that all corp stops are open? Yes / No					
Verify that pressure gauge is mounted at high point of line to be tested? Yes / No If no, correct for elevation difference (ie. add 0.433 psi per foot elevation difference).					
System Static Pressure (psi):	Starting Pressure (psi): (greater of 150 psi or 1.5 times static)	Ending Pressure (psi):			
Pipe Lengths & φ's:	Starting Time:	Ending Time (2 hours minimum):			
Volume Required to Reach Initial Test Pressure (gal):	Allowable Leakage (gal): (2 times table or calculated value below)	Measured Leakage (gal):			
TEST RESULTS: Pass / Fail					

ALLOWABLE LEAKAGE PER 1,000 FEET OF PIPELINE - gph (NOTE: double the values from table below for a 2 hour test)

Test Pressure psi		NOMINAL PIPE DIAMETER - in.								
<i>p.</i>	3	4	6	8	10	12	14	16	18	20
200	0.32	0.43	0.64	0.85	1.06	1.28	1.48	1.70	1.91	2.12
175	0.30	0.40	0.59	0.80	0.99	1.19	1.39	1.59	1.79	1.98
150	0.28	0.37	0.55	0.74	0.92	1.10	1.29	1.47	1.66	1.84

If the pipeline under test contains various diameters, the allowable leakage shall be the sum of the allowable leakage for each size.

No additional leakage allowance will be given for fire hydrant assemblies or valves.

Sample: 700' 8" and 55' 6" pipe. $\rightarrow \rightarrow 0.74 \text{ gph} / 1,000' * 700') + (0.55 \text{ gph} / 1,000' * 55') = 0.548 \text{ gph} * 2 \text{ hours} = ~1.1 \text{ gallon allowable leakage loss.}$

Allowable leakage based on : $L = SD(P)^{1/2}/133,200$

Where:

L = allowable leakage, in gallons per hour

D = nominal diameter of the pipe, in inches

S = length of pipe tested, in feet

P = test pressure during the leakage test, in psig

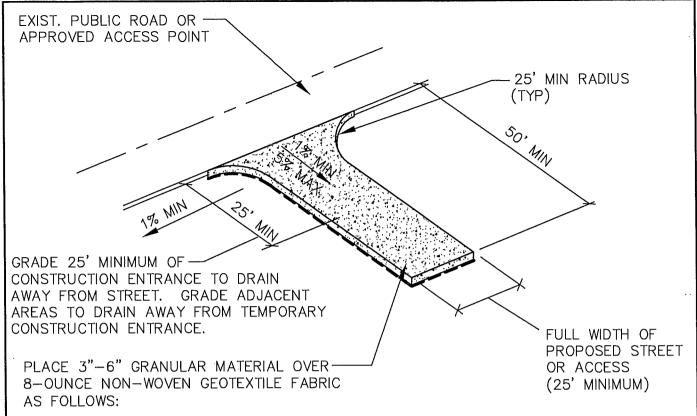
Regardless of leakage, maximum pressure drop during test period shall not exceed 5 psi over the 2 hour test period.

Any visible leaks shall be repaired regardless of the whether or not the pipeline meets leakage allowance.

TEST PROCEDURE

- 1. Apply hydrostatic pressure by pumping water from an auxiliary supply basin. Accurately determine the amount of water required to reach the initial test pressure by refilling the supply basin with a calibrated container following pressurization of pipeline.
- 2. Monitor test pressure for 2 hour period.
- 3. At the completion of the test period, re-pressurize the pipeline by pumping water from the auxiliary supply basin (mark the water surface level in the auxiliary supply basin prior to re-pressurization).
- 4. Accurately determine the amount of water required to reach the test pressure by refilling the supply basin to the marked line with a calibrated container following re-pressurization of pipeline. If the measured leakage is less than the allowable leakage, the test is successful.

Reference: For summary of disinfection & bacteriological testing procedures, see construction notes under Appendix B.



DRY WEATHER ACCESS

14-INCH MIN. DEPTH OVER COMPACTED SUBGRADE & FABRIC

WET WEATHER ACCESS

24-INCH MIN. DEPTH OVER UNDISTURBED SUBGRADE & FABRIC

CONSTRUCTION NOTES:

- 1. THE AREA OF THE CONSTRUCTION ENTRANCE SHALL BE STRIPPED OF ALL TOPSOIL, VEGETATION, ROOTS, AND OTHER NON-COMPACTABLE MATERIAL.
- 2. SUBGRADE SHALL BE COMPACTED AND PROOFROLLED PRIOR TO PLACEMENT OF GRANULAR MATERIAL. FAILURE TO PASS PROOFROLL WILL REQUIRE USE OF WET WEATHER SECTION.
- 3. FAILURE OR PUMPING OF THE DRY WEATHER SECTION WILL REQUIRE REMOVAL OF THE GRANULAR MATERIAL AND INSTALLATION OF THE WET WEATHER SECTION.

MAINTENANCE NOTES:

1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOW OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE PERIODIC

TOP DRESSING WITH 3"-6" INCH STONE AS CONDITIONS DEMAND, AND REPAIR AND/OR CLEAN-OUT OF STRUCTURES USED TO TRAP SEDIMENT.

 ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.

3. ALL TRUCKS TRANSPORTING SATURATED SOILS SHALL BE WELL SEALED. WATER DRIPPAGE FROM TRUCKS MUST BE REDUCED TO 1 GALLON PER HOUR PRIOR TO LEAVING THE SITE.

TEMPORARY
CONSTRUCTION
ENTRANCE
(NTS)
DAYTON, OR

AMAY 2013

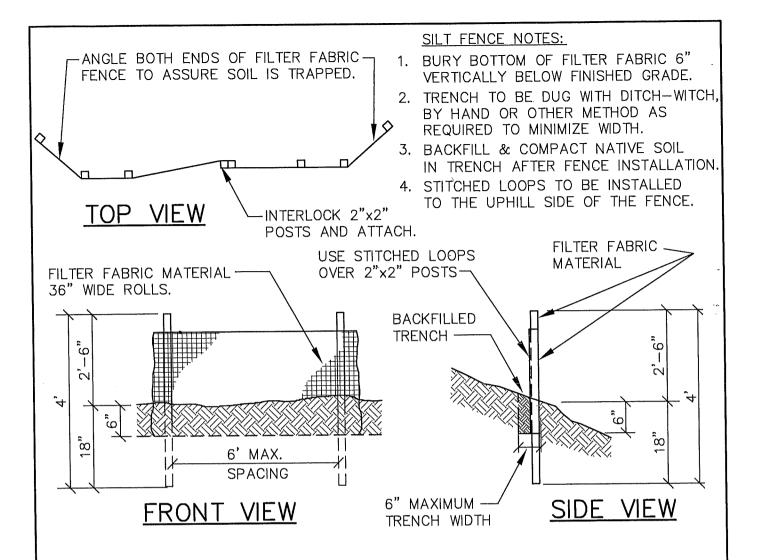
STANDARD

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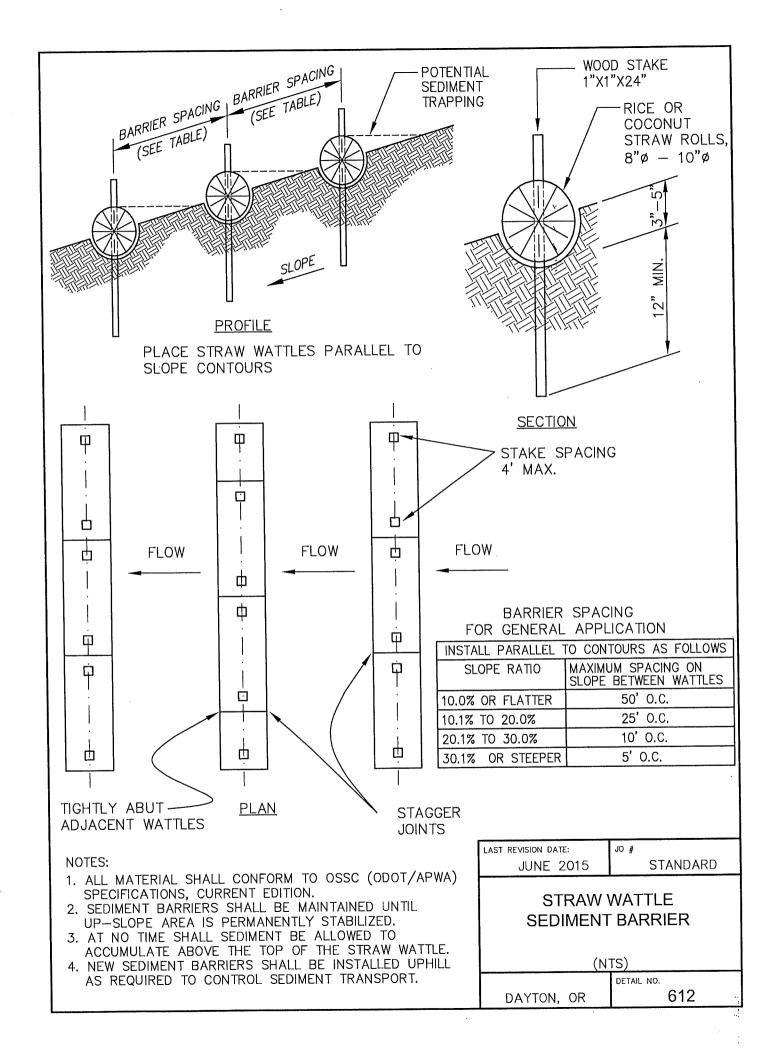
STANDARD

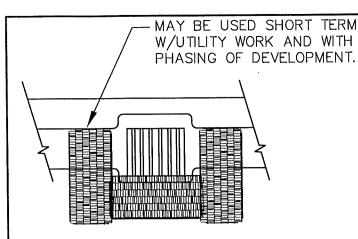


MAINTENANCE NOTES:

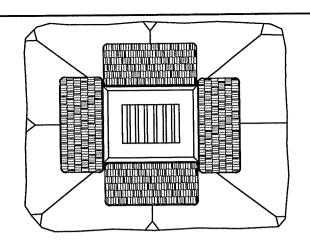
- 1. SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED.
- 2. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE BEHIND SEDIMENT FENCES OR BIOFILTER BAGS.
- 3. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.

LAST REVISION DATE:	JO #
APRIL 2014	STANDARD
SEDIMENT	BARRIERS
	DETAIL NO.
DAYTON, OR	611

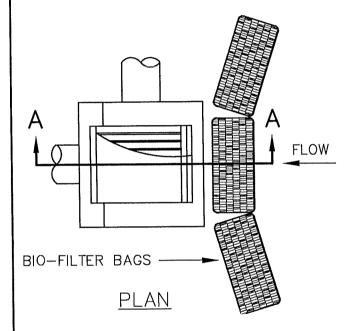


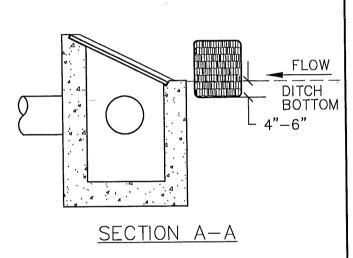






AREA DRAIN



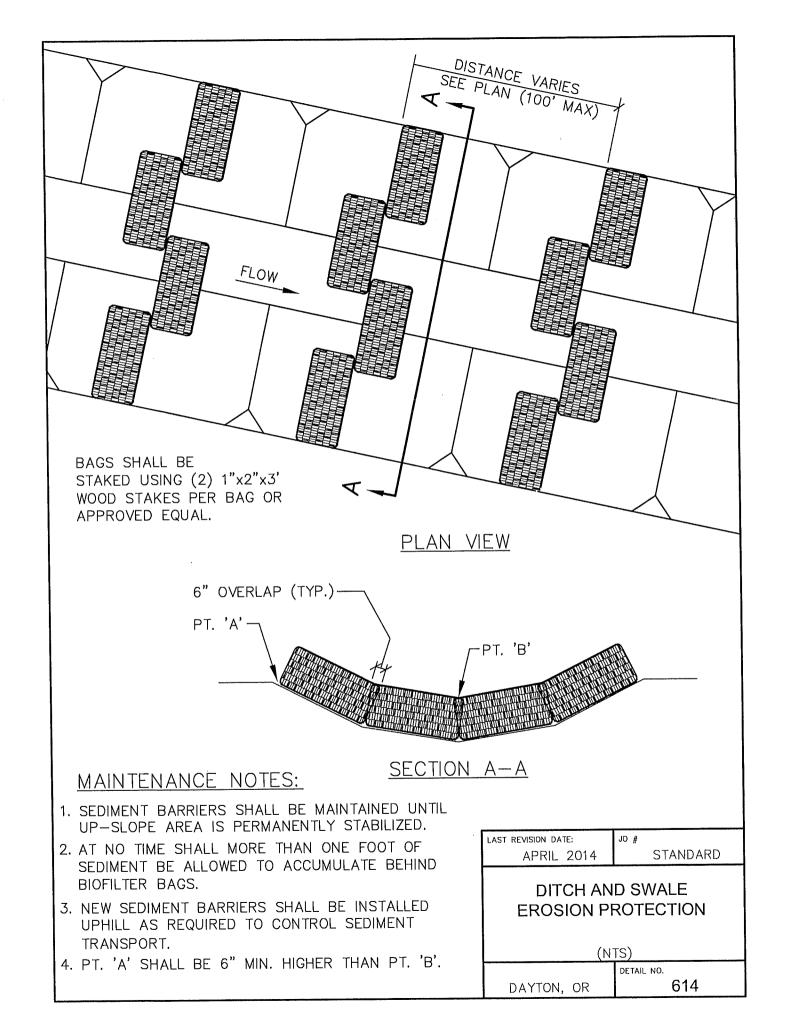


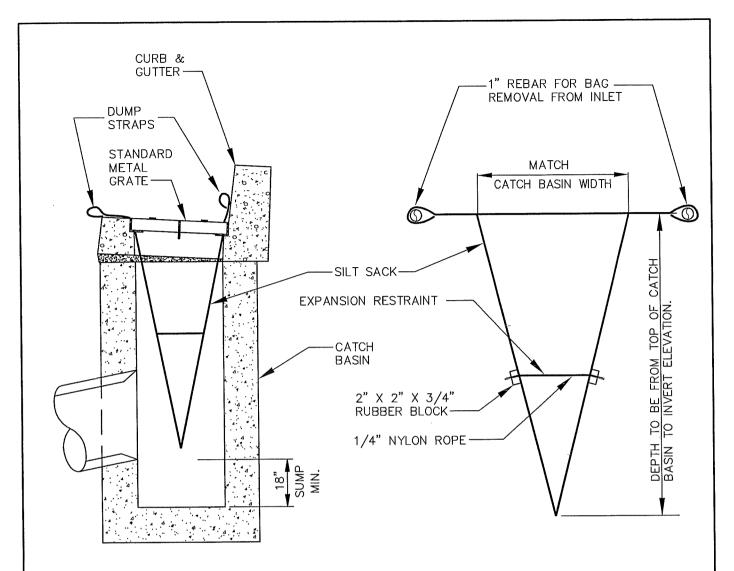
DITCH INLET C.B.

MAINTENANCE NOTES:

- 1. SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED.
- 2. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE BEHIND SEDIMENT FENCES OR BIOFILTER BAGS.
- 3. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.

LAST REVISION DATE: APRIL 2014	JO # STANDARD		
INLET SEDIMENT CONTROL (NTS)			
DAYTON, OR	DETAIL NO. 613		



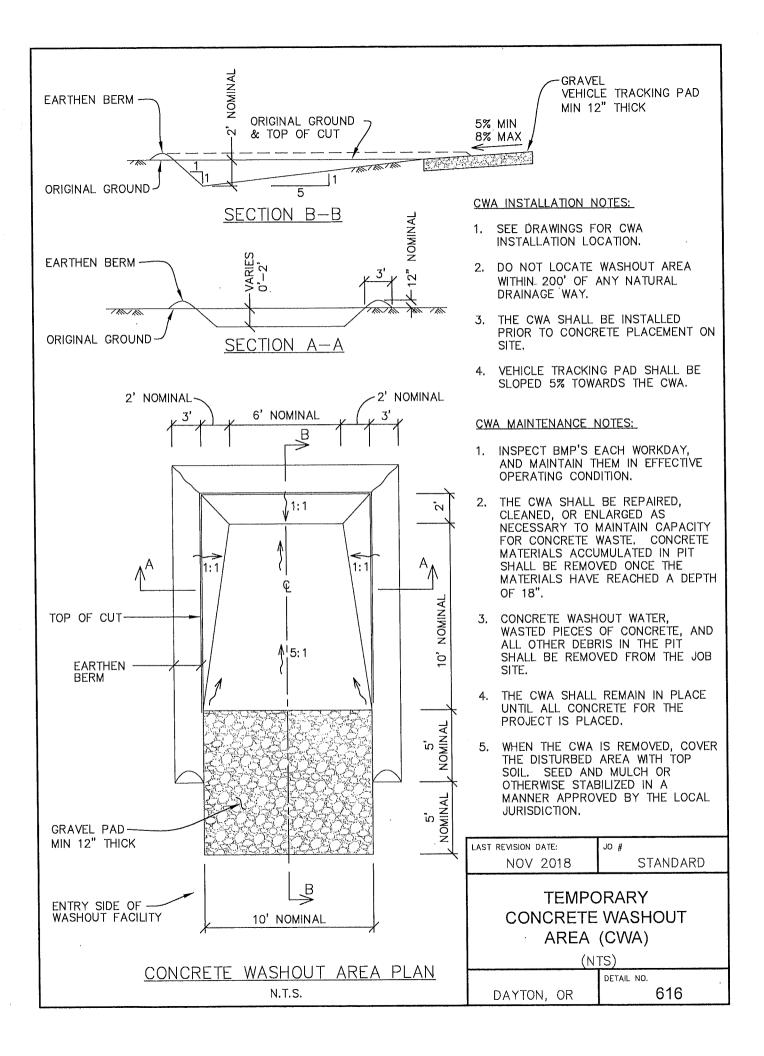


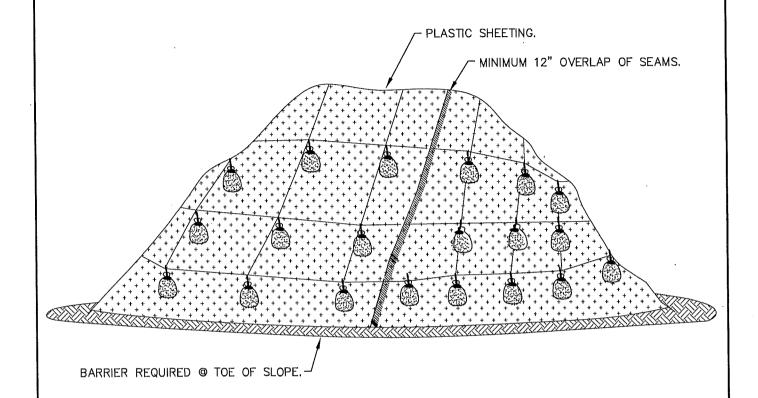
INSTALLATION DETAIL

BAG DETAIL

- 1. EMPTY SILT SACK AS NECESSARY.
- 2. SILTSACK SEDIMENT CONTROL DEVICE AS MANUFACTURED BY ACF ENVIRONMENTAL AND SUPPLIED BY ACF WEST (503) 771-5115 OR APPROVED EQUAL.

LAST REVISION DATE: SEPT 2006			
SILT SACK INLET DETAIL (NTS)			
DAYTON, OR	DETAIL NO. 615		





STOCKPILE DETAIL

- 1. MINIMUM 12" OVERLAP OF ALL SEAMS REQUIRED.
- 2. SEDIMENT BARRIER REQUIRED @ TOE OF STOCK PILE.
- 3. COVERING MAINTAINED TIGHTLY IN PLACE BY USING SANDBAGS OR TIRES ON ROPES WITH A MAXIMUM 10' GRID SPACING IN ALL DIRECTIONS.
- 4. PLASTIC SHEETING TO EXTEND A MINIMUM OF 12" PAST THE BOTTOM OF THE PILE ONTO SURROUNDING GRADE ON ALL SIDES.

LAST REVISION DATE:	JO #	
JAN 2019	STANDARD	
STOCKPILE COVER DETAIL		
(NTS)		
DAYTON, OR	DETAIL NO. 617	
DATION, OR	<u> </u>	